

<u>MERIDIAN</u>

ASSUMED

<u>LEGEND:</u>

BFNC BOARD FENCE
PP POWER POLE
PPU POWER POLE W/UNDERGROUND
PTU POWER POLE W/XFMR&UG
WM WATER METER
AP APPLE
C CEDAR
D DECIDUOUS
LOC LOCUST
P PINE
CD CALCULATED DIMENSION
M MEASURED DIMENSION

	ASPHALT HATCH		ROCKERY
44	CONCRETE HATCH		CONIFER(AS NOTED) DECIDUOUS(AS NOTED)
4 .	DECK HATCH	XOHP	FENCE LINE AS NOTED OVERHEAD POWER LINE

CONTOUR INTERVAL = 2'

BENCHMARK & DATUM INFO

VERICAL DATUM: NAVD88

ORIGINAL BM: 2 1/2" DIA. IRON PIPE WITH INVERTED NAIL IN CASE ON W MERCER WAY. GSOW ID BM-11081.

ELEV. = 92.88

GENERAL NOTES

1. THE INFORMATION DEPICTED ON THIS MAP REPRESENTS THE RESULTS OF A SURVEY MADE ON THE DATE INDICATED AND CAN ONLY BE CONSIDERED AS INDICATING THE GENERAL CONDITION EXISTING AT THAT TIME.

2. UNDERGROUND UTILITIES WERE LOCATED BASED ON THE SURFACE EVIDENCE OF UTILITIES (I.E. PAINT MARKS, SAW CUTS IN PAVEMENT, COVERS, LIDS ETC.) THE CONTRACTOR SHALL VERIFY THE EXACT LOCATION, ELEVATION AND SIZE OF EXISTING UTILITIES PRIOR TO CONSTRUCTION.

3. TREE SIZES WERE LOCATED & SPECIES DETERMINED TO THE BEST OF OUR ABILITY. HOWEVER, TYEE SURVEYORS DOES NOT WARRANT THE ACCURACY OF SIZE & SPECIES SHOWN HEREON. ANY TREES CONSIDERED TO BE CRITICAL SHOULD BE VERIFIED BY A TRAINED

- 4. THIS MAP DOES NOT PURPORT TO SHOW EASEMENTS OF RECORD, IF ANY.
- 5. NO PROPERTY CORNERS WERE SET IN CONJUNCTION WITH THIS SURVEY.
- 6. THE INTENT OF THIS SURVEY IS TO AID IN DESIGN/PLANNING FOR PARCELS SHOWN.

7. THE BOUNDARY FOR THESE SITES WAS COMPUTED FROM RECORDS OF SURVEY NO'S. 9610189001, 20070614900001, 20160408900001, 9709109005, 9709109005, AND FIELD MEASUREMENTS.

8. GARAGE FINISH FLOOR = 48.95 GARAGE RIDGE HEIGHT - 63.10

9. UTILITY AND DRIVEWAY EASEMENT (AFN#20200820001842) PLOTS AT THE SAME LOCATION AS EASEMENT SHOWN ON CITY OF MERCER ISLAND SUBDIVISION, AS RECORDED MAY 29TH, 1963.

LEGAL DESCRIPTION

SITE "A'

THAT PORTION OF GOVERNMENT LOT 4, SECTION 7, TOWNSHIP 24 NORTH, RANGE 5 EAST, W.M., IN KING COUNTY, WASHINGTON, DESCRIBED AS FOLLOWS:

BEGINNING AT A POINT ON THE SOUTH LINE OF SAID GOVERNMENT LOT WHICH POINT IS NORTH 89°57'00" WEST 726.00 FEET FROM THE SOUTHEAST CORNER THEREOF, AS SHOWN ON THE ORIGINAL PLAT OF LAKEMONT, ACCORDING TO THE UNRECORDED PLAT THEREOF, (SAID SOUTHEAST CORNER BEING NORTH 89°57'00" WEST, 1,333.64 FEET FROM THE SOUTHEAST CORNER OF GOVERNMENT LOT 5, IN SAID SECTION 7); THENCE NORTH 1230.0 FEET TO THE TRUE POINT OF BEGINNING OF THIS DESCRIPTION: THENCE SOUTH 89°57'00" EAST 80.00 FEET; THENCE NORTH 20.00 FEET TO A POINT CALLED HEREIN "X" THENCE CONTINUING NORTH 153.00 FEET; THENCE NORTH 89°57'00 WEST 80 FEET TO A POINT FROM WHICH THE TRUE POINT OF BEGINNING BEARS SOUTH; THENCE 153.00 FEET TO THE POINT OF BEGINNING; TOGETHER WITH AN EASEMENT FOR DRIVEWAY AND UTILITY PURPOSES OVER A 20 FOOT WIDE STRIP, THE WEST LINE OF WHICH BEGINS AT POINT "X" ABOVED DESCRIBED AND RUNS SOUTH 160 FEET.

THAT PORTION OF GOVERNMENT LOT 4, SECTION 7, TOWNSHIP 24 NORTH, RANGE 5 EAST, W.M., IN KING COUNTY, WASHINGTON, DESCRIBED AS FOLLOWS:

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THENCE NORTH 1070 FEET TOT THE TRUE POINT OF BEGINNING; THENCE CONTINUING NORTH 140.00 FEET;

THENCE CONTINUING NORTH 140.00 FEE THENCE NORTH 89'57'00" WEST 80.00

THENCE SOUTH 140.00 FEET;
THENCE SOUTH 89'57'00" EAST 80.00 FEET TO THE TRUE POINT OF BEGINNING;

(ALSO BEING KNOWN AS A PORTION OF TRACTS 57 AND 58 IN REPLAT OF TRACTS E,F,G,H,I,J, AND K OF LAKEMONT, AN UNRECORDED PLAT.)

SITUATED IN THE CITY OF MERCER ISLAND, COUNTY OF KING, STATE OF WASHINGTON.

EQUIPMENT & PROCEDURES

FIELD SURVEY CONDUCTED USING A COMBINATION OF GPS USING A REFERENCE NETWORK AND A 5" ELECTRONIC TOTAL STATION WAS USED FOR THIS FIELD TRAVERSE SURVEY.

SURVEY PROCEDURES MEET OR EXCEED STATE STANDARDS AS SPECIFIED BY W.A.C. 332-130 WITH REGARD TO LINEAR AND ANGULAR CLOSURES.

ALL MEASURING INSTRUMENTS FOR THIS SURVEY HAVE BEEN MAINTAINED ACCORDING TO

ALL MEASURING INSTRUMENTS FOR THIS SURVEY HAVE BEEN MAINTAINED ACCORDING TO MANUFACTURES SPECIFICATIONS AND HAVE BEEN COMPARED WITH A NATIONAL GEODETIC SURVEY CALIBRATED BASELINE WITHIN THE LAST 12 MONTHS.

REVISED: 2/6/22 - UTILITY AND DRIVEWAY EASEMENT ADDED. RG

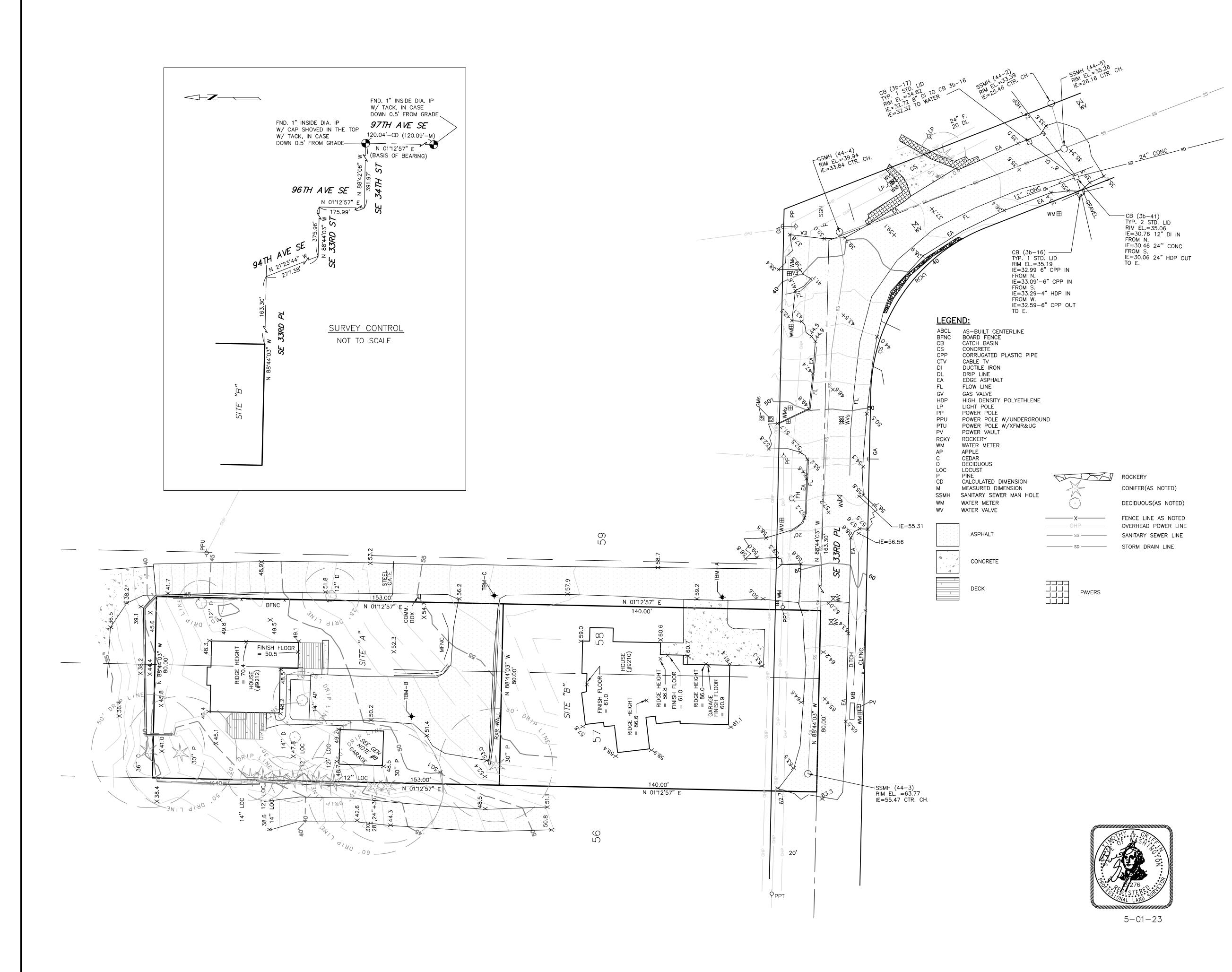
NW1/4, SE1/4, SEC. 7, T. 24 N., R. 4 E., W.M. MERCER ISLAND, WASHINGTON

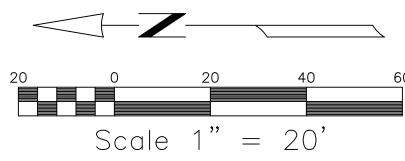
BOUNDARY/TOPOGRAPHIC SURVEY	
for	

DEBRA SCHATZMAN

. /	Tyee Surve Rofessional land enwood av. n. seattle, w	ŚURVEYORS
DRAWN BY:	DATE:	JOB NO.:
RG	6-10-19	19080
CHKD BY:	SCALE:	SHEET:
TG	1" = 20'	1 OF

9210 SE 33RD PL MERCER ISLAND, WASHINGTON 98040





MERIDIAN

ASSUMED

CONTOUR INTERVAL = 2'

BENCHMARK & DATUM INFO

VERICAL DATUM: NAVD88

ORIGINAL BM: 2 1/2" DIA. IRON PIPE WITH INVERTED NAIL IN CASE ON W MERCER WAY. GSOW ID BM-11081.

ELEV. = 92.88TBM - A: SET MAG NAIL. ELEV. = 59.75

SET MAG NAIL. ELEV. = 51.00SET MAG NAIL. ELEV. = 57.05

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- 8. GARAGE FINISH FLOOR = 48.95 GARAGE RIDGE HEIGHT 63.10
- 9. DESIGNATIONS FOR CATCH BASINS AND SEWER MANHOLES FROM CITY OF MERCER ISLAND GIS. IT APPEARS THAT THE STORM PIPES, AS SHOWN ON CITY OF MERCER ISLAND GIS ARE INCORRECT. PIPE DIRECTIONS SHOWN HERON ARE FROM FIELD OBSERVATIONS. MANHOLES MAY OR MAY NOT HAVE ADDITIONAL PIPE INVERTS. ONLY PIPES THAT ARE VISIBLE FROM TOP OF STRUCTURE ARE MEASURED AND SHOWN.

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> PARCEL NO.: 4139300316 NW1/4, SE1/4, SEC. 7, T. 24 N., R. 5 E., W.M. MERCER ISLAND, WASHINGTON

> > TOPOGRAPHY SURVEY

BILL PLUMMER

9212 SE 33RD PL

MERCER ISLAND, WASHINGTON 98040

	Tyee Surve ROFESSIONAL LAND LE AVE N., SUITE 107, SHORELIN	ŚURVEYORS
DRAWN BY:	DATE:	JOB NO.:
NP	3-20-2023	21252
CHKD BY:	SCALE:	SHEET:
TG	1" = 20'	1 OF

NW1/4, SE1/4, SEC. 7, T. 24 N., R. 4 E., W.M.

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EROSION AND SEDIMENT CONTROL NOTES

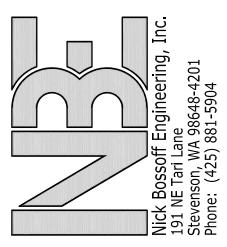
- APPROVAL OF THIS EROSION AND SEDIMENT CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAINAGE DESIGN (E.G., SIZE AND LOCATION OF ROADS, PIPES, RESTRICTORS, CHANNELS, RETENTION FACILITIES, UTILITIES.
- 2. THE IMPLEMENTATION OF THESE ESC PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND UPGRADING OF THESE ESC FACILITIES IS THE RESPONSIBILITY OF THE APPLICANT/ESC SUPERVISOR UNTIL ALL CONSTRUCTION IS APPROVED.
- THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED BY A CONTINUOUS LENGTH OF SURVEY TAPE (OR FENCING, IF REQUIRED) PRIOR TO CONSTRUCTION. DURING THE CONSTRUCTION PERIOD, NO DISTURBANCE BEYOND THE CLEARING LIMITS SHALL BE PERMITTED. THE CLEARING LIMITS SHALL BE MAINTAINED BY THE APPLICANT/ESC SUPERVISOR FOR THE DURATION OF CONSTRUCTION.
- 4. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED PRIOR TO OR IN CONJUNCTION WITH ALL CLEARING AND PROPERTIES IS MINIMIZED.
- THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD, THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND MODIFIED TO ACCOUNT FOR CHANGING SITE CONDITIONS (E.G., ADDITIONAL SUMP PUMPS, RELOCATION OF DITCHES AND SILT FENCES, ETC.).
- THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/ESC SUPERVISOR AND MAINTAINED TO ENSURE CONTINUED PROPER FUNCTIONING. WRITTEN RECORDS SHALL BE KEPT OF WEEKLY REVIEWS OF THE ESC FACILITIES DURING THE WET SEASON (OCT. 1 TO APRIL 30) AND OF MONTHLY REVIEWS DURING THE DRY SEASON (MAY 1 TO SEPT. 30)
- ANY AREAS OF EXPOSED SOILS, INCLUDING ROADWAY EMBANKMENTS, THAT WILL NOT BE DISTURBED FOR TWO DAYS DURING THE WET SEASON OR SEVEN DAYS DURING THE DRY SEASON SHALL BE IMMEDIATELY STABILIZED WITH THE APPROVED ESC METHODS (E.G., SEEDING, MULCHING, PLASTIC COVERING, ETC.)
- ANY AREA NEEDING ESC MEASURES NOT REQUIRING IMMEDIATE ATTENTION SHALL BE ADDRESSED WITHIN FIFTEEN (15) DAYS. 9. THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH OR WITHIN
- 10. AT NO TIME SHALL MORE THAN ONE (1) FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN. ALL CATCH
- BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAVING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER INTO THE DOWNSTREAM SYSTEM.
- 11. STABILIZED CONSTRUCTION ENTRANCES AND ROADS SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES, SUCH AS WASH PADS, MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT.
- 12. ANY PERMANENT FLOW CONTROL FACILITY USED AS A TEMPORARY SETTLING BASIN SHALL BE MODIFIED WITH THE NECESSARY EROSION CONTROL MEASURES AND SHALL PROVIDE ADEQUATE STORAGE CAPACITY. IF THE FACILITY IS TO FUNCTION ULTIMATELY AS AN INFILTRATION SYSTEM. THE TEMPORARY FACILITY MUST BE GRADED SO THAT THE BOTTOM AND SIDES ARE AT LEAST THREE FEET ABOVE THE FINAL GRADE OF THE PERMANENT FACILITY.
- 13. WHERE STRAW MULCH FOR TEMPORARY EROSION CONTROL IS REQUIRED, IT SHALL BE APPLIED AT A MINIMUM THICKNESS OF 2 TO 3 INCHES.
- CAN BE SEEDED IN PREPARATION FOR THE WINTER RAINS. DISTURBED AREAS SHALL BE SEEDED WITHIN ONE WEEK OF THE BEGINNING OF THE WET SEASON. A SKETCH MAP OF THOSE AREAS TO BE SEEDED AND THOSE AREAS TO REMAIN UNCOVERED SHALL BE SUBMITTED TO THE DDES INSPECTOR. THE DDES INSPECTOR CAN REQUIRE SEEDING OF ADDITIONAL AREAS IN ORDER TO PROTECT SURFACE WATERS, ADJACENT PROPERTIES, OR DRAINAGE FACILITIES.

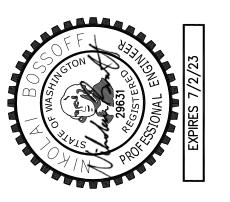
POLLUTION PREVENTION AND SPILL CONTROL

STORAGE AND HANDLING OF LIQUIDS

MINIMIZE AMOUNT OF LIQUIDS STORED ON SITE.

- STORE AND CONTAIN LIQUID MATERIALS IN SUCH A MANNER THAT IF A VESSEL IS RUPTURED OR LEAKS, THE CONTENTS WILL NOT DISCHARGE, FLOW, OR BE WASHED INTO THE STORM DRAINAGE SYSTEM, SURFACE WATERS, OR GROUNDWATER. TYPICALLY THIS MEANS INSTALLING SECONDARY CONTAINMENT, SUCH AS A LINED EXCAVATION, LARGER CONTAINER, OR USING A DOUBLE-WALLED TANK OR SIMILAR COMMERCIALLY AVAILABLE CONTAINMENT FACILITY. PLACE TIGHT-FITTING LIDS ON ALL CONTAINERS.
- ENCLOSE OR COVER THE CONTAINERS WHERE THEY ARE STORED TO PROTECT FROM RAIN. THE LOCAL FIRE DISTRICT MUST BE CONSULTED FOR LIMITATIONS ON CLEARANCE OF ROOF COVERS OVER CONTAINERS USED TO STORE FLAMMABLE MATERIALS.
- RAISE THE CONTAINERS OFF THE GROUND BY USING A SPILL CONTAINMENT PALLET OR SIMILAR METHOD THAT HAS PROVISIONS FOR SPILL CONTROL.
- PLACE DRIP PANS OR ABSORBENT MATERIALS BENEATH ALL MOUNTED CONTAINER TAPS. AND AT ALL POTENTIAL DRIP AND SPILL LOCATIONS DURING FILLING AND UNLOADING OF CONTAINERS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED. RECYCLED. OR PROPERLY DISPOSED OF.
- STORE AND MAINTAIN ABSORBENT PADS OR APPROPRIATE SPILL CLEANUP MATERIALS NEAR THE CONTAINER STORAGE AREA, IN A LOCATION KNOWN TO ALL. ENSURE THAT EMPLOYEES ARE FAMILIAR WITH THE SITE'S SPILL PLAN AND/OR PROPER SPILL CLEANUP
- 8. CHECK CONTAINERS (AND ANY CONTAINMENT SUMPS) DAILY FOR LEAKS AND SPILLS. REPLACE CONTAINERS THAT ARE LEAKING. CORRODED. OR OTHERWISE DETERIORATING. IF THE LIQUID CHEMICALS ARE CORROSIVE, CONTAINERS MADE OF COMPATIBLE MATERIALS MUST BE USED INSTEAD OF METAL DRUMS. NEW OR SECONDARY CONTAINERS MUST BE LABELED WITH THE PRODUCT
- 9. PLACE DRIP PANS OR ABSORBENT MATERIALS BENEATH A CONTAINER THAT IS FOUND TO BE LEAKING. REMOVE THE DAMAGED CONTAINER AS SOON AS POSSIBLE. MOP UP THE SPILLED LIQUID WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.
- 1. LOCATE THE FUELING OPERATION TO ENSURE LEAKS OR SPILLS WILL NOT DISCHARGE, FLOW, OR BE WASHED INTO THE STORM
- DRAINAGE SYSTEM, SURFACE WATER, OR GROUNDWATER. USE DRIP PANS OR ABSORBENT PADS TO CAPTURE DRIPS OR SPILLS DURING FUELING OPERATIONS.
- 3. IF FUELING IS DONE DURING EVENING HOURS, LIGHTING MUST BE PROVIDED.
- 4. STORE AND MAINTAIN APPROPRIATE SPILL CLEANUP MATERIALS IN THE MOBILE FUELING VEHICLE. ENSURE THAT EMPLOYEES ARE FAMILIAR WITH PROPER SPILL CONTROL AND CLEANUP PROCEDURES.
- 5. IMMEDIATELY MOP UP ANY SPILLED FUEL WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.
- CONCRETE SAW CUTTING, SLURRY, AND WASHWATER DISPOSAL SLURRY FROM SAW CUTTING THE SIDEWALK SHALL BE VACUUMED SO THAT IT DOES NOT ENTER NEARBY STORM DRAINS.
- CONCRETE TRUCK CHUTES, PUMPS, AND INTERNALS SHALL BE WASHED OUT ONLY INTO FORMED AREAS AWAITING INSTALLATION OF CONCRETE.
- 3. UNUSED CONCRETE REMAINING IN THE TRUCK AND PUMP SHALL BE RETURNED TO THE ORIGINATING BATCH PLANT FOR RECYCLING.
- 4. HAND TOOLS INCLUDING, BUT NOT LIMITED, SCREEDS, SHOVELS, RAKES, FLOATS, AND TROWELS SHALL BE WASHED OFF ONLY INTO FORMED INTO FORMED AREAS AWAITING INSTALLATION OF CONCRETE OR IMPERMEABLE ASPHALT.
- EQUIPMENT THAT CANNOT BE EASILY MOVED, SUCH AS CONCRETE PAVERS, SHALL ONLY BE WASHED IN AREAS THAT DO NOT DIRECTLY DRAIN TO NATURAL OR CONSTRUCTED STORMWATER CONVEYANCES.
- WASHDOWN FROM AREAS SUCH AS CONCRETE AGGREGATE DRIVEWAY SHALL NOT DRAIN DIRECTLY TO NATURAL OR CONSTRUCTED STORMWATER CONVEYANCES.
- WHEN NO FORMED AREAS ARE AVAILABLE, WASHWATER AND LEFTOVER PRODUCT SHALL BE CONTAINED IN A LINED CONTAINER. CONTAINED CONCRETE SHALL BE DISPOSED OF IN A MANNER THAT DOES NOT VIOLATE GROUNDWATER OR SURFACE WATER
- 8. CONTAINERS SHALL BE CHECKED FOR HOLES IN THE LINER DAILY DURING CONCRETE POURS AND REPLACED THE SAME DAY.





PERMIT SUBMITTAL	CITY REVISIONS	DETENTION/PUMP ADDED	CITY COMMENTS			
7	77	23	/23			

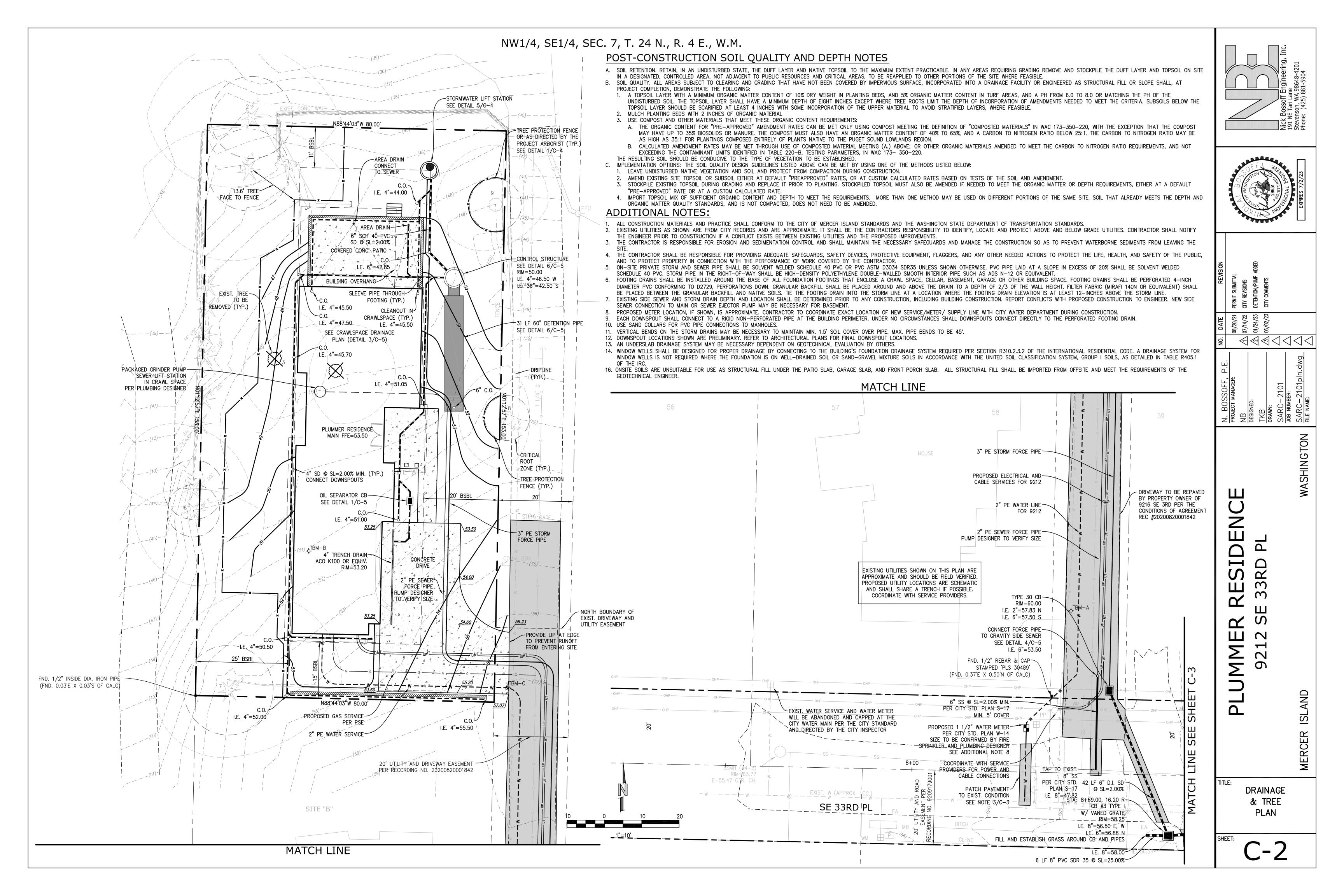
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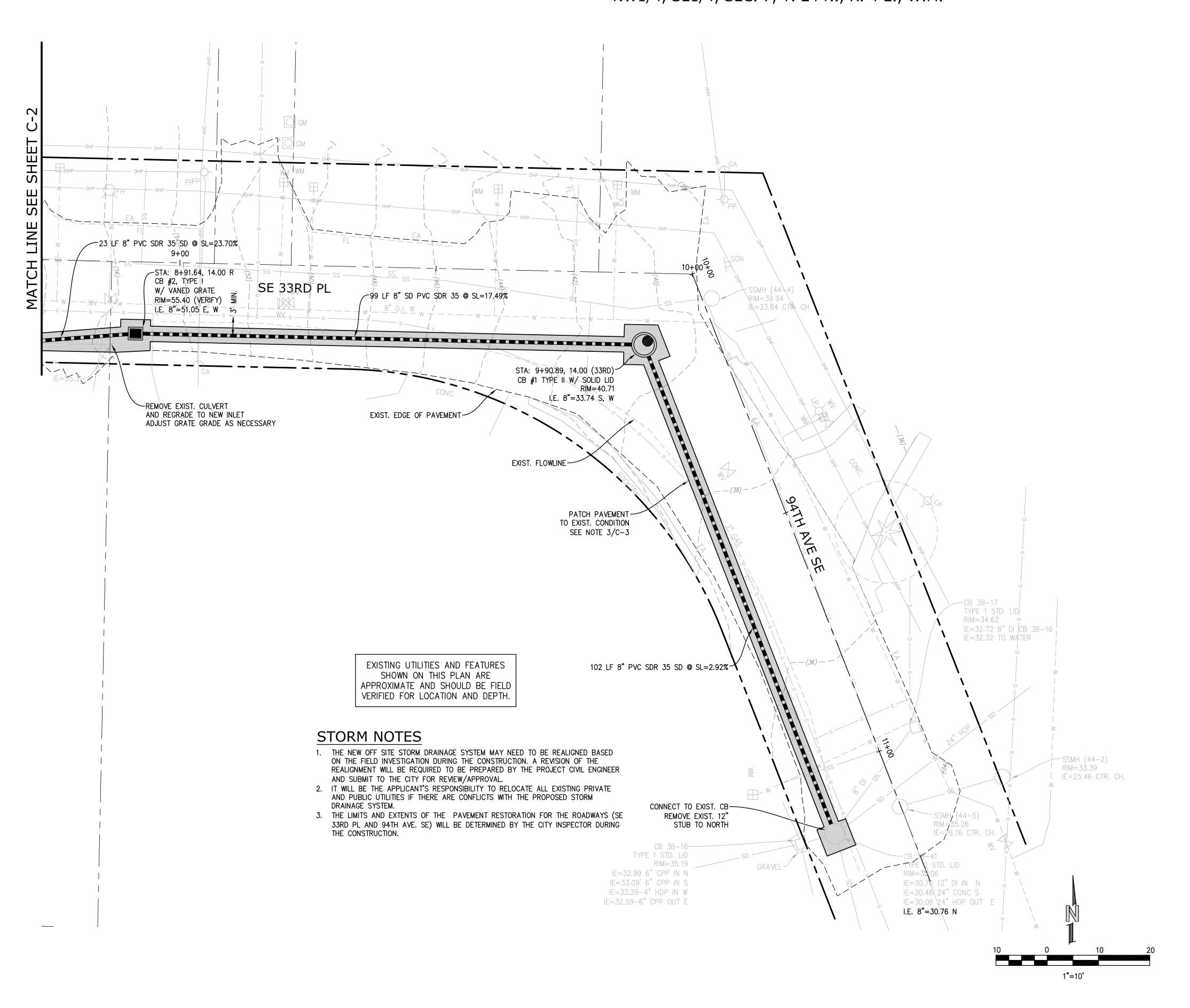
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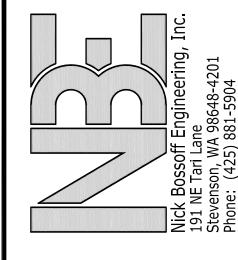
Д 9

T.E.S.C. PLAN

CALL 48 HOURS BEFORE YOU DIG 1-800-424-5555







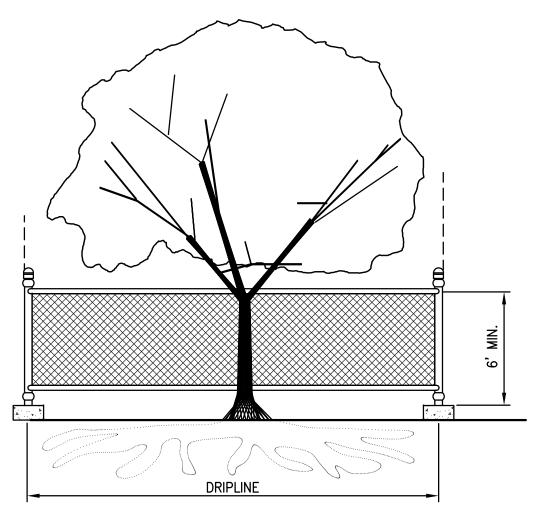


150 100 100 100 100 100 100 100 100 100	STATE ON WASHING	2 29631 O	PASSONAL ENGINE	EXPIRES 7/2/23

	PROJECT MANAGER:		08/20/21	Permit Submittal
	NB	\triangleleft	\triangle 07/14/22	CITY REVISIONS
	DESIGNED:	(N 104/93	DETENTION /PIIMP ADDED
	TKB	1 <	27/27/20	
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DRAINAGE PLAN



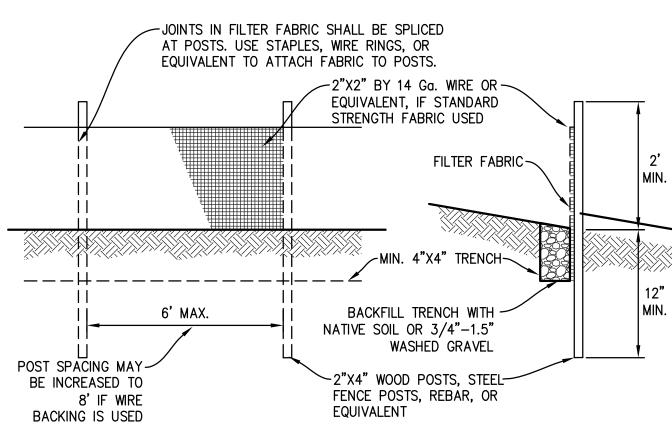
TREE PROTECTION DURING CONSTRUCTION

- 1. 6-FT. HIGH TEMPORARY CHAIN LINK FENCE SHALL BE PLACED AT THE DRIPLINE OF THE TREE TO BE SAVED. FENCE SHALL COMPLETELY ENCIRCLE THE TREE(S). INSTALL FENCE POSTS USING PIER BLOCKS ONLY. AVOID DRIVING POSTS OR STAKES INTO MÁJOR ROOTS.
- 2. FOR ROOTS OVER 1-IN DIA. THAT ARE DAMAGED DURING CONSTRUCTION, MAKE A CLEAN, STRAIGHT CUT TO REMOVE THE DAMAGED PORTION: ALL EXPOSED ROOTS SHALL BE TEMPORARILY COVERED WITH DAMP BURLAP TO PREVENT DRYING, AND SHALL BE COVERED WITH SOIL AS SOON AS POSSIBLE
- 3. WORK WITHIN PROTECTION FENCE SHALL BE DONE MANUALLY. NO STOCKPILING OF MATERIALS, VEHICULAR TRAFFIC, OR STORAGE OF EQUIPMENT OR MACHINERY SHALL BE ALLOWED WITHIN THE LIMIT OF THE FENCING.

TREE PROTECTION

SCALE: NTS

∼ANTI-BUOYANCY BASE



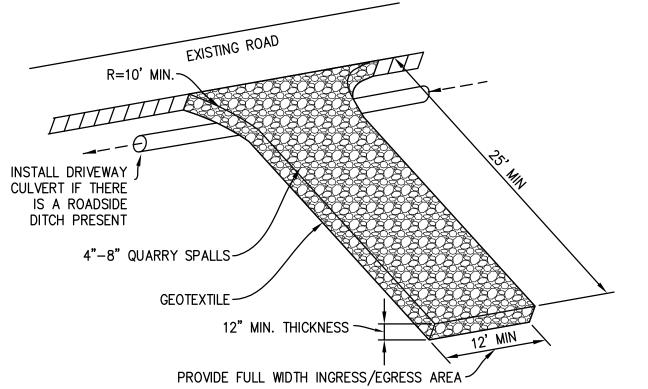
NOTE: FILTER FABRIC FENCE SHALL BE INSTALLED ALONG CONTOUR WHENEVER POSSIBLE.

MAINTENANCE STANDARDS

- ANY DAMAGE SHALL BE REPAIRED IMMEDIATELY.
- 2. IF CONCENTRATED FLOWS ARE EVIDENT UPHILL OF THE FENCE, THEY MUST BE INTERCEPTED AND CONVEYED TO A SEDIMENT TRAP OR POND.
- 3. IT IS IMPORTANT TO CHECK THE UPHILL SIDE OF THE FENCE FOR SIGN OF THE FENCE CLOGGING AND ACTING AS A BARRIER TO FLOW AND THEN CAUSING CHANNELIZATION OF FLOWS PARALLEL TO THE FENCE. IF THIS OCCUR, REPLACE THE FENCE AND/OR REMOVE THE TRAPPED SEDIMENT.
- 4. SEDIMENT MUST BE REMOVED WHEN THE SEDIMENT IS 6" HIGH.
- 5. IF THE FILTER FABRIC HAS DETERIORATED DUE TO ULTRAVIOLET BREAKDOWN, IT SHALL BE REPLACED.

SILT FENCE

SCALE: NTS

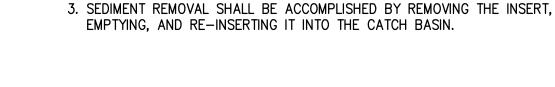


MAINTENANCE STANDARDS

- 1. QUARRY SPALLS (OR HOG FUEL) SHALL BE ADDED IF THE PAD IS NO LONGER IN ACCORDANCE WITH THE SPECIFICATIONS.
- 2. IF THE ENTRANCE IS NOT PREVENTING SEDIMENT FROM BEING TRACKED ONTO PAVEMENT, THEN ALTERNATIVE MEASURES TO KEEP THE STREETS FREE OF SEDIMENT SHALL BE USED. THIS MAY INCLUDE STREET SWEEPING, AN INCREASE IN THE DIMENSIONS OF THE ENTRANCE, OR THE INSTALLATION OF A WHEEL WASH. IF WASHING IS USED, IT SHALL BE DONE ON AN AREA COVERED WITH CRUSHED ROCK, AND WASH WATER SHALL DRAIN TO A SEDIMENT TRAP OR POND.
- 3. ANY SEDIMENT THAT IS TRACKED ONTO PAVEMENT SHALL BE REMOVED IMMEDIATELY BY SWEEPING. THE SEDIMENT COLLECTED BY SWEEPING SHALL BE REMOVED OR STABILIZED ON—SITE. THE PAVEMENT SHALL NOT BE CLEANED BY WASHING DOWN THE STREET, EXCEPT WHEN SWEEPING IS INEFFECTIVE AND THERE IS A THREAT TO PUBLIC SAFETY. IF IT IS NECESSARY TO WASH THE STREET, THE CONSTRUCTION OF A SMALL SUMP SHALL BE CONSIDERED. THE SEDIMENT WOULD THEN BE WASHED INTO THE SUMP.
- 4. ANY ROCK SPALLS THAT ARE LOOSENED FROM THE PAD AND END UP ON THE ROADWAY SHALL BE REMOVED IMMEDIATELY.
- 5. IF VEHICLES ARE ENTERING OR EXITING THE SITE AT POINTS OTHER THAN THE CONSTRUCTION ENTRANCE(S), FENCING (SECTION 5.4.1) SHALL BE INSTALLED TO CONTROL TRAFFIC.

ROCK CONSTRUCTION ENTRANCE

SCALE: NTS



ADAPTOR SKIRT-

TRIM TO WITHIN 3" - 5" OF GRATE

GEOTEXTILE -

FABRIC

UPON PLACEMENT OF A NEW CATCH BASIN.

1. INSERT SHALL BE INSTALLED PRIOR TO CLEARING AND GRADING ACTIVITY, OR

2. SEDIMENT SHALL BE REMOVED FROM THE UNIT WHEN IT BECOMES HALF FULL.

CB INSERT

SCALE: NTS

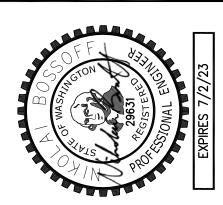
OVERFLOW BYPASS

FOR PEAK STORM

ACCUMULATION

VOLUMES

-SEDIMENT



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DETAILS

2'ø SOLID LOCKING -~ 2'ø SOLID LOCKING COVER COVER W/ 4 @ 1" VENT HOLES THERMOPLASTIC JUNCTION BOX, — SHOWN ROTATED OUT OF TRUE POSITION EL = 45.83WATERTIGHT SEAL (TYP.) --2" GATE VALVE STAINLESS STEEL FLOAT -BRACKET, SHOWN ROTATED OUT OF TRUE POSITION - 2" PVC UNION DISCHARGE 2" CHECK VALVE I.E. 6"=42.40 LEAD PUMP ON EL=42.40 48" DIA. CONC. MANHOLE SIZE TO BE VERIFIED W/ PUMP SUPPLIER - HEAVY DUTY LIFT CHAIN. ATTATCH TO TOP OF MANHOLE HEAVY DUTY -LIFT CHAIN. ATTATCH TO — 2" PVC PIPE TOP OF EL=40.40 CLEARANCE PER MANHOLE MANUFACTURER TO PREVENT FLOAT HANG-UP 2 @ ALTERNATING MYER WHRE-PUMPS OR EQUIV, W/ 2" DISCHARGE. MIN CAPACITY OF 45 GPM @ 22.0' HEAD. EL=38.40 18"x18" 4" CONC. PAD-

-GALVANIZED STEPS OR LADDER 48" DIA. CONC. MANHOLE SIZE TO BE VERIFIED W/ PUMP SUPPLIER TO ACCOMMODATE PUMPS ACCESS -COVER ABOVE I.E. 6"=42.40 2" PVC√ CONCRETE BLOCKING (TYP.) 3" FORCE MAIN-

PVC SCH 40

-FLOAT BRACKET 2" ELECTRICAL HUB

-THE SYSTEM IS TO BE AN ALTERNATING DUPLEX SYSTEM. -LOCATE CONTROL PANEL AND ALARM ON EXTERIOR BUILDING WALL. USE HYDROMATIC PANEL OR APPROVED EQUIVALENT.

-SYSTEM TO BE FULLY AUTOMATIC WITH MANUAL OVERRIDE. -ALARM TO BE AUDIO (BELL) AND VISUAL (LIGHT). -BOTH PUMPS TO OPERATE AT "LAG PUMP ON" FLOAT

-SCH 80 PVC PIPE INSIDE MANHOLE. -FOLLOW MANUFACTURER'S INSTRUCTIONS FOR ALL INSTALLATION.

-PROVIDE ELECTRICAL SUPPLY TO PANEL AND LIFT STATION PER MANUFACTURER'S SPECIFICATIONS. POWER TO PANEL AND PUMP SHALL BE ON A DEDICATED CIRCUIT. -ELECTRICAL CONNECTIONS AND SERVICES WITHIN THE PUMP WETWELL SHOULD BE WATERTIGHT.

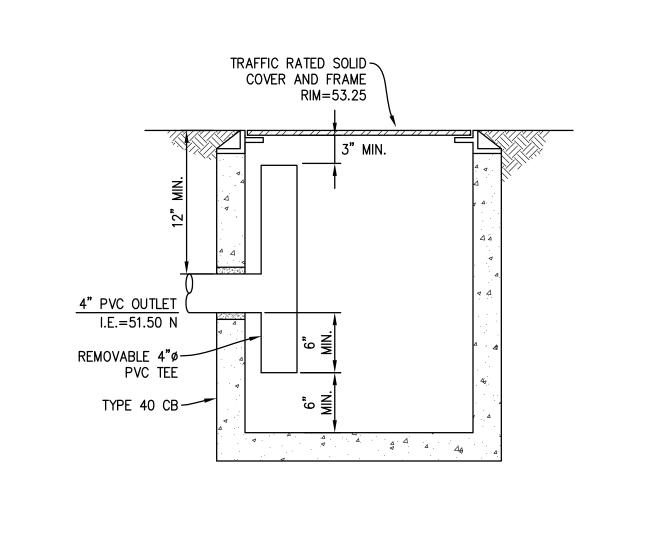
-THE PRIVATE PROPERTY OWNER(S) SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM.

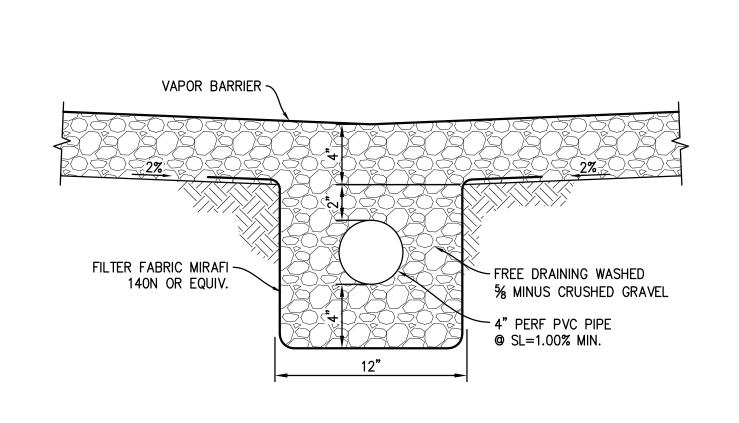
-PUMP SYSTEMS SHALL BE OWNED, OPERATED, MAINTAINED, REPAIRED, AND REPLACED (AS NEEDED) BY PROPERTY OWNER(S) SERVED BY SUCH SYSTEM. THE PUMP SYSTEM SHALL HAVE DUAL, ALTERNATING PUMPS WITH EMERGENCY ON-SITE, BACK-UP POWER SUPPLY AND AN EXTERNAL ALARM SYSTEM FOR SYSTEM FAILURES. IT IS THE SOLE RESPONSIBILITY OF THE HOME OWNER IF FLOODING OR SEWER BACKUP OCCURS DUE TO THE FAILURE OF THE PUMP SYSTEM. IT IS THE RESPONSIBILITY OF THE HOME OWNER TO PROVIDE AN ADEQUATE AND FUNCTIONAL BACKUP SYSTEM FOR THE PUMP SYSTEM IN THE EVENT OF

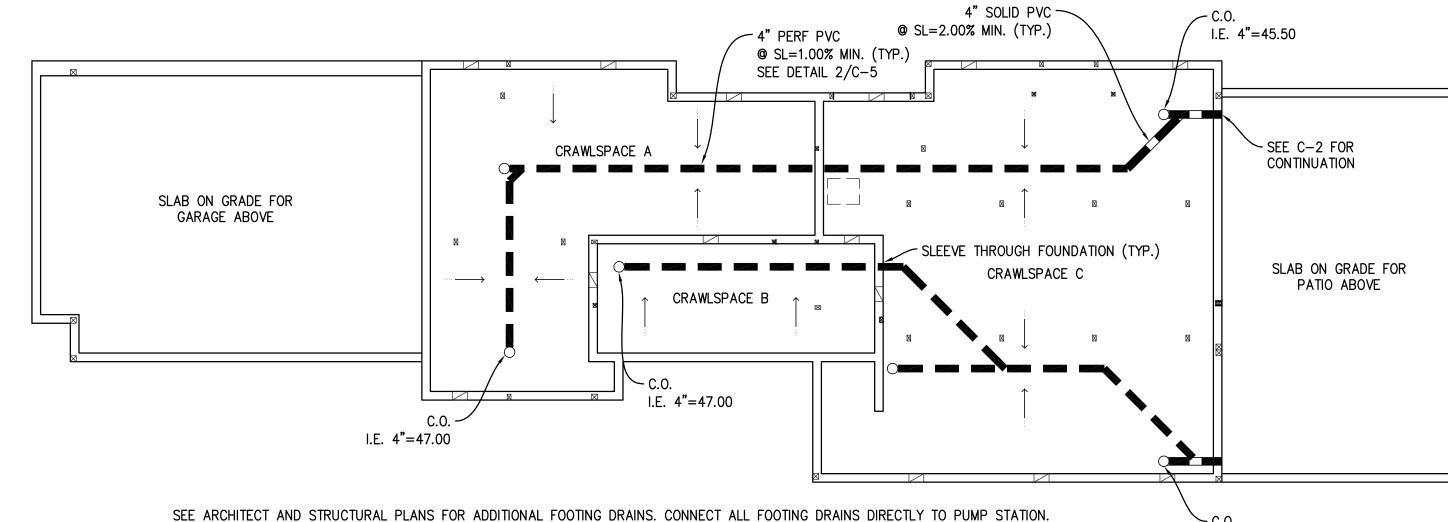
THE POWER FAILURE.

DRAIN LIFT STATION #1

SCALE: NTS







OIL SEPARATOR CB SCALE: NTS

CRAWL SPACE DRAIN SCALE: NTS

CRAWL SPACE DRAINAGE

SCALE: NTS

SCALE: NTS

I.E. 4"=45.50

ALUMINUM PIPE, OR REINFORCED CONCRETE PIPE. CORRUGATED STEEL PIPE IS NOT ALLOWED.

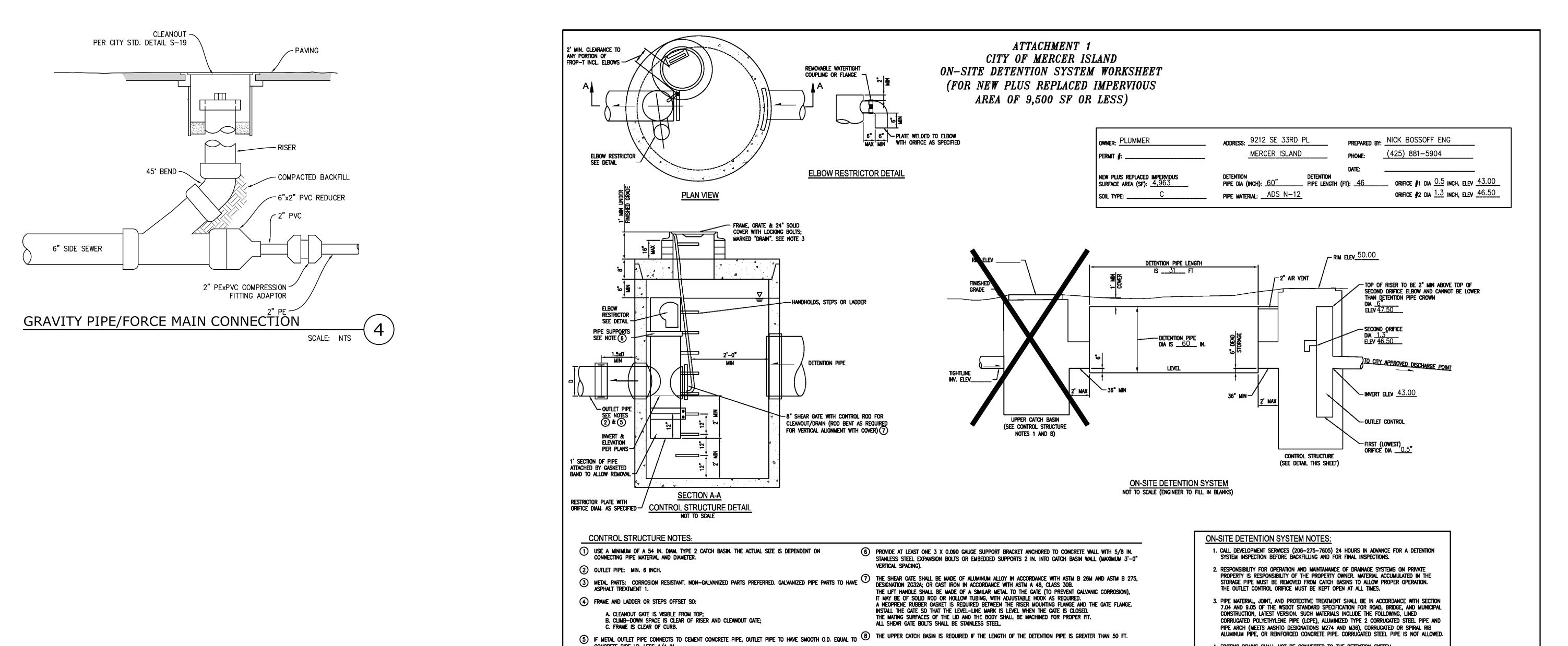
4. FOOTING DRAINS SHALL NOT BE CONNECTED TO THE DETENTION SYSTEM.

INGTON

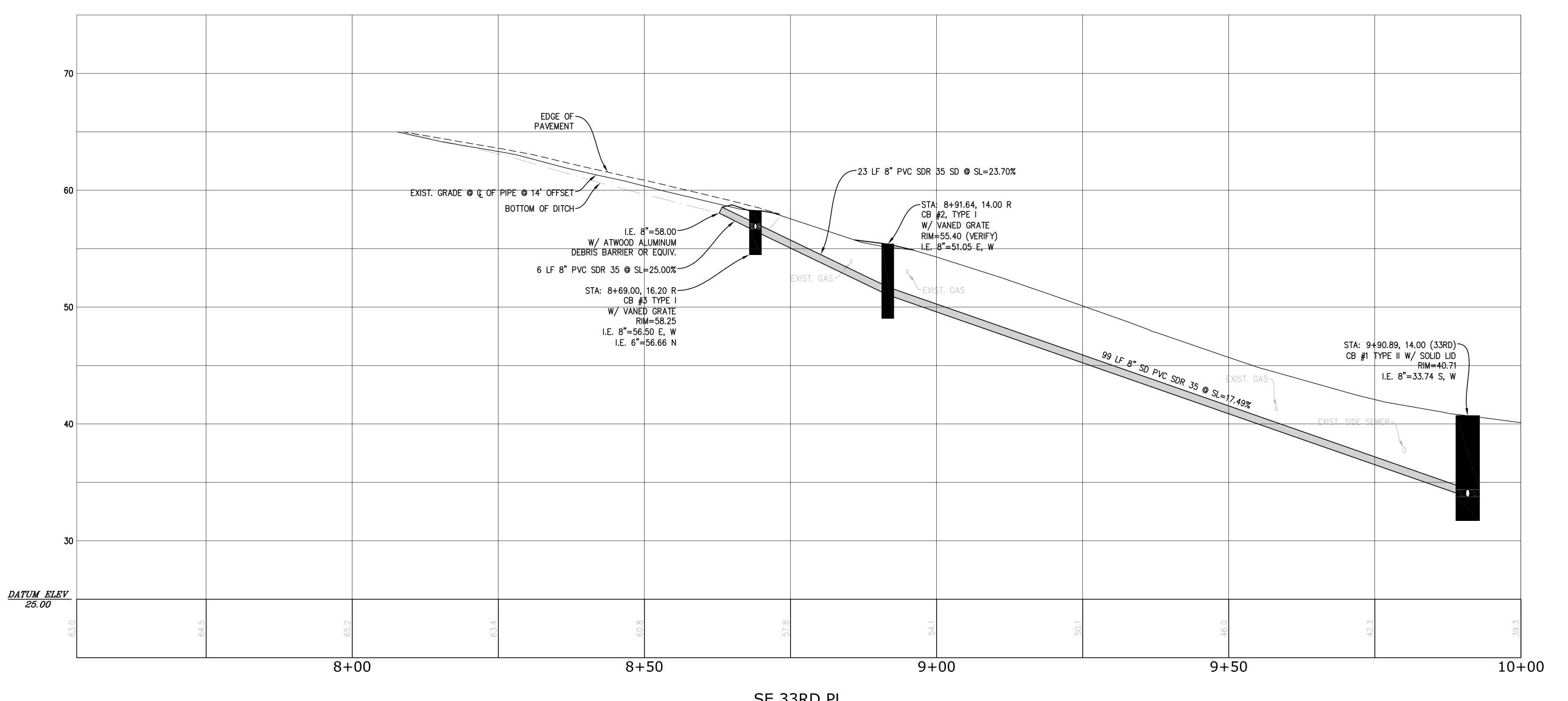
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DETAILS

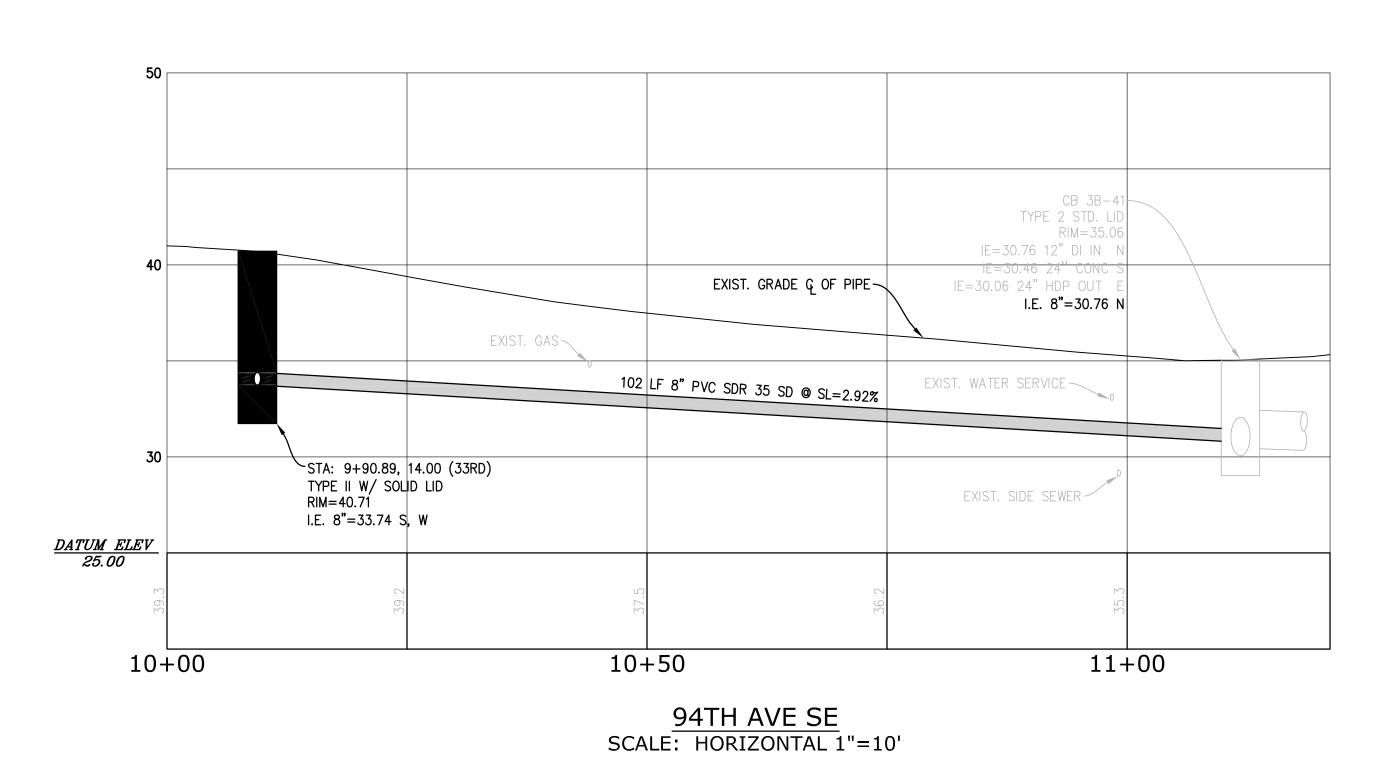
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CONCRETE PIPE I.D. LESS 1/4 IN.

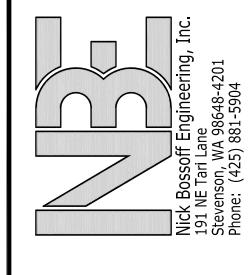


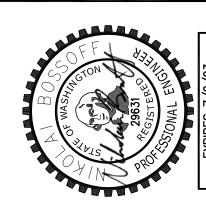
SE 33RD PL SCALE: HORIZONTAL 1"=10' VERTICAL 1"=5'



VERTICAL 1"=5'

ALL EXISTING UTILITIES SHALL BE
POTHOLED AT PROPOSED STORM
ALIGNMENT AND ELEVATIONS DETERMINED.
CONFLICTS SHALL BE REPORTED TO
ENGINEER PRIOR TO CONSTRUCTION.





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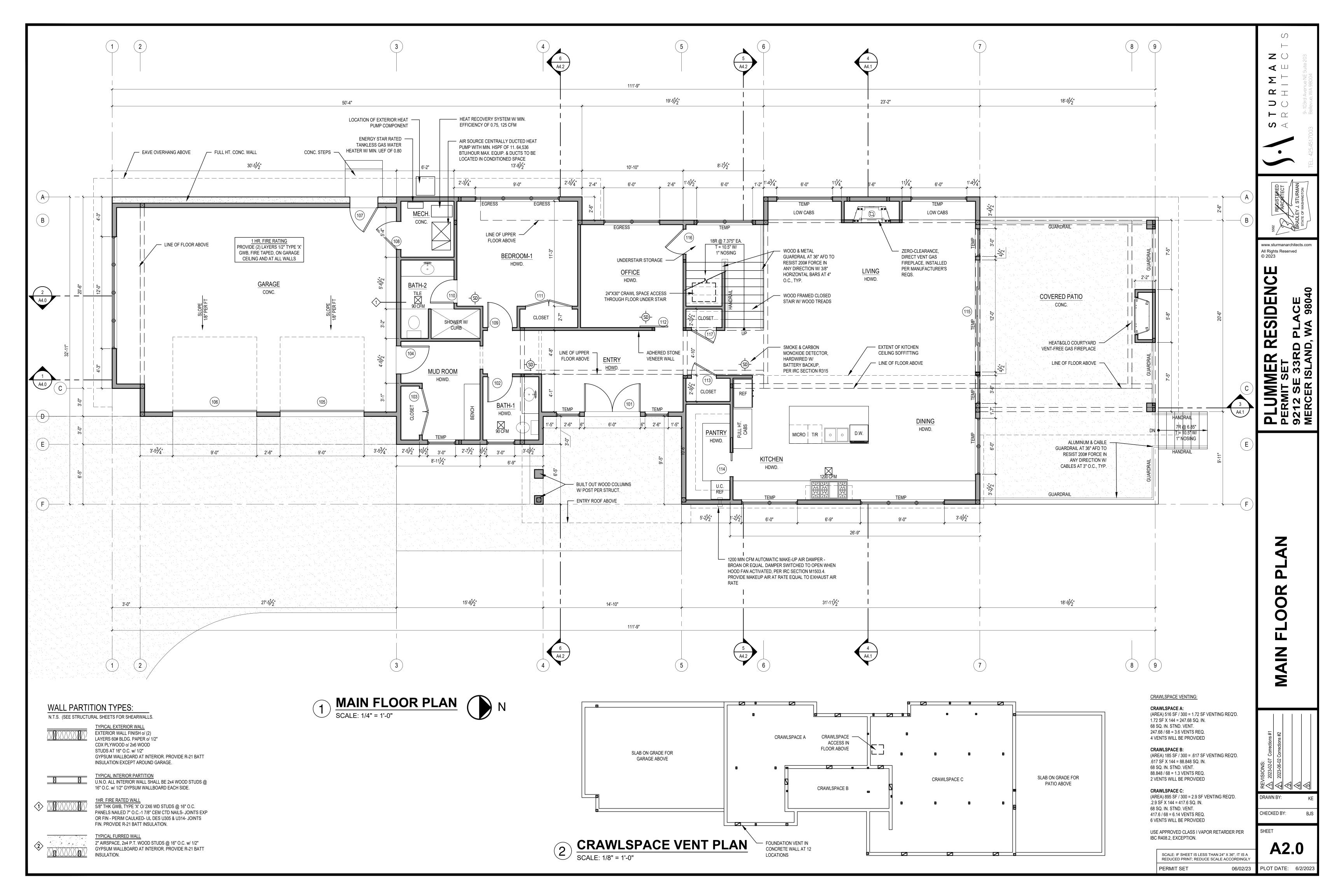
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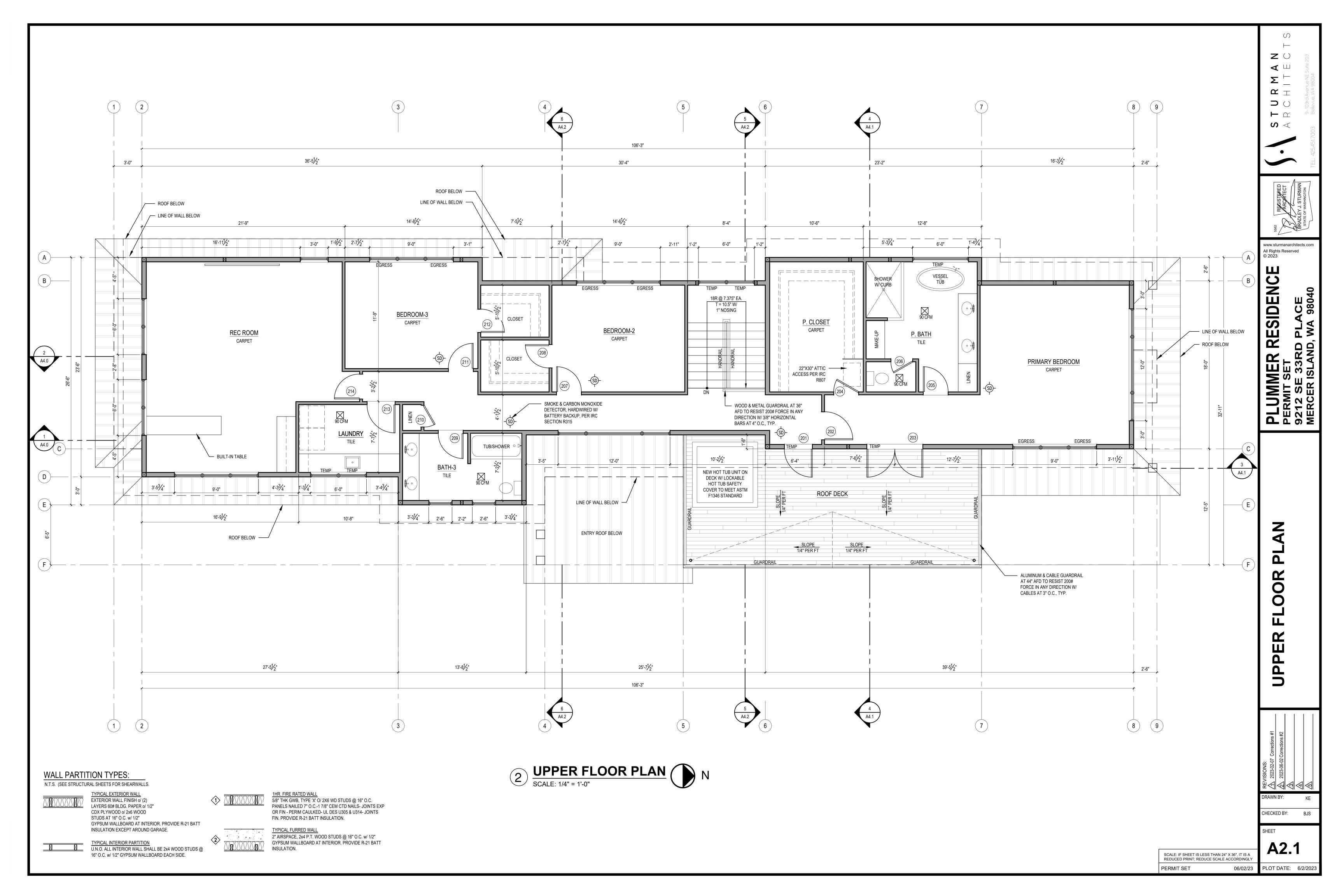
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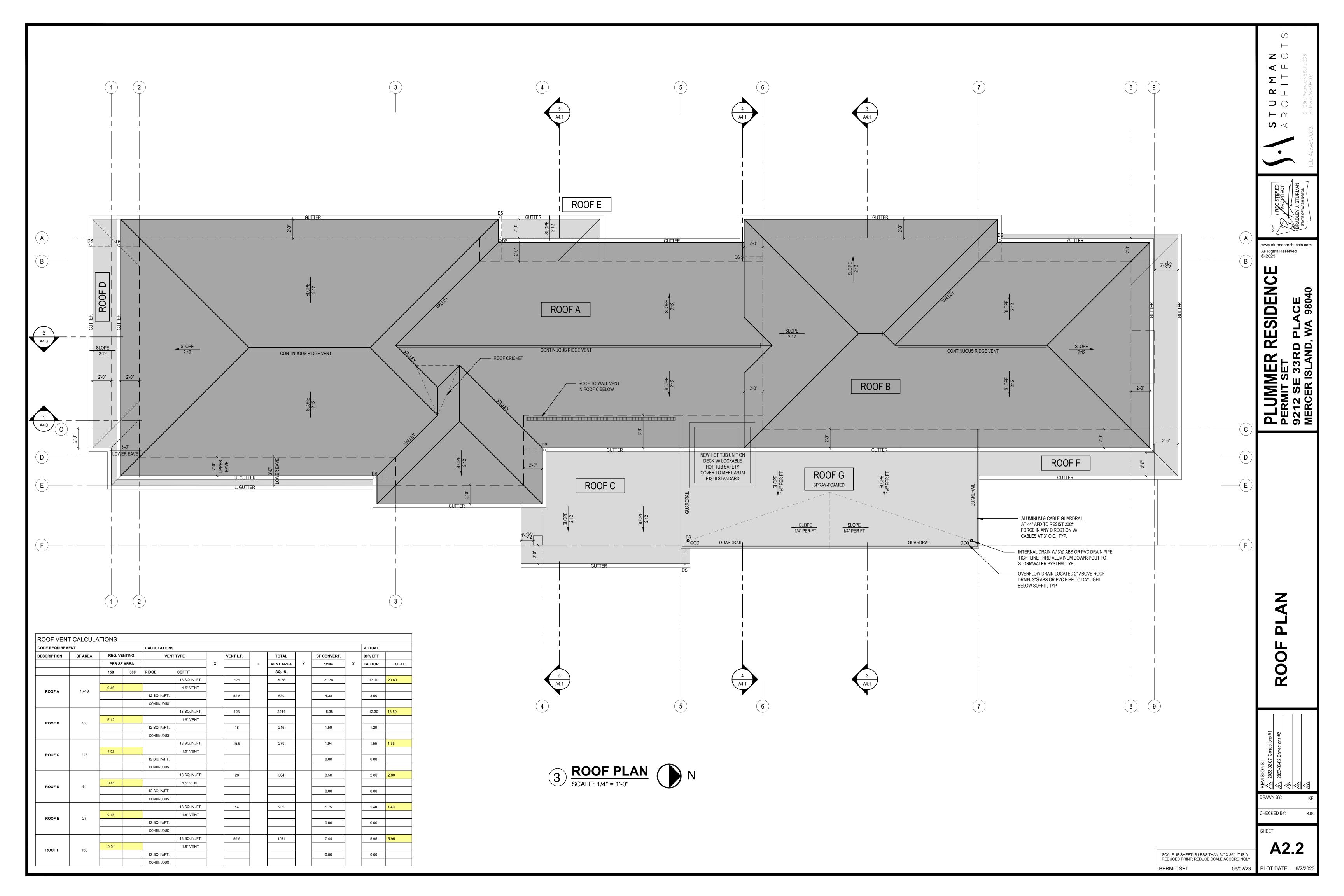
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MERCER ISLAND

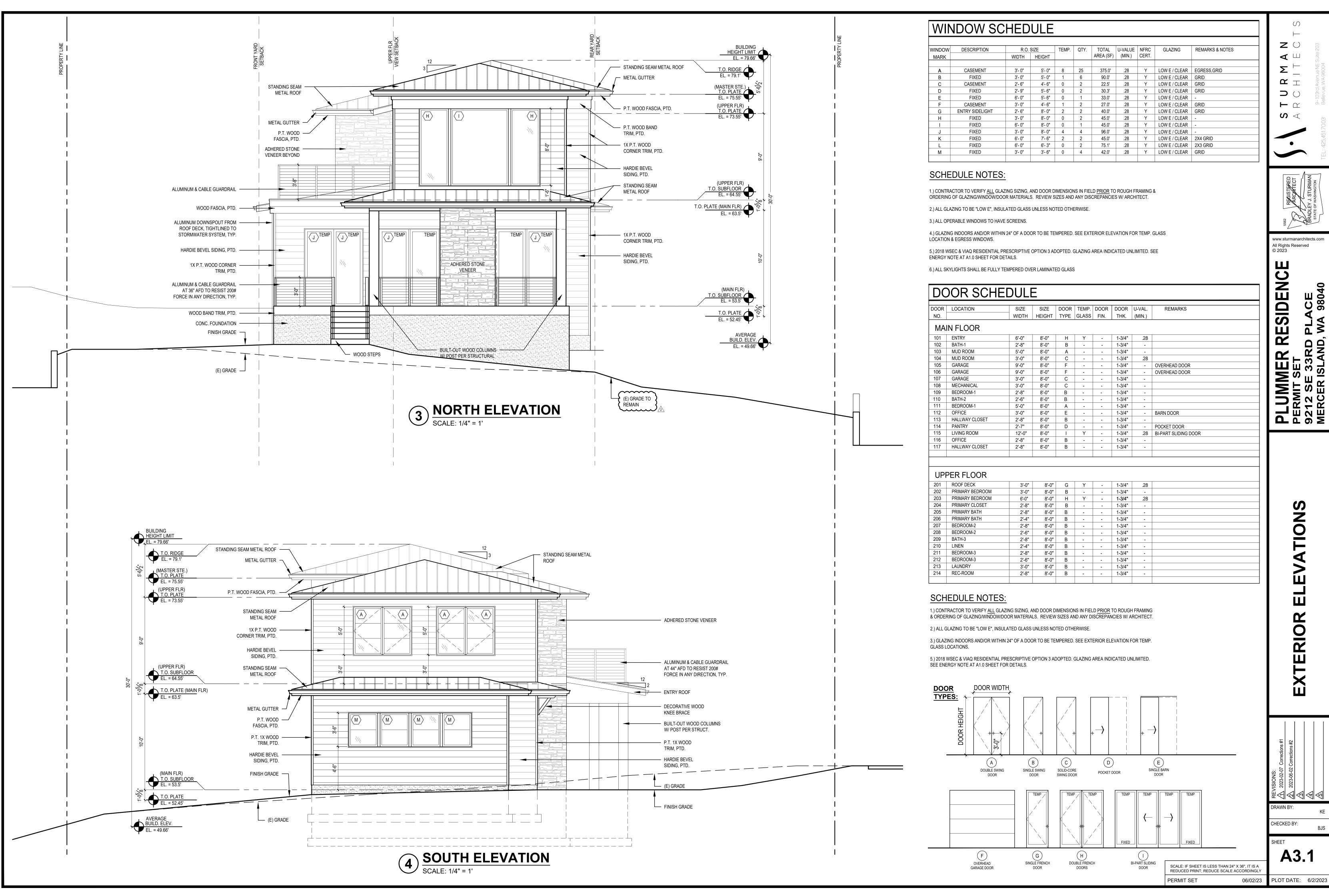
PROFILES

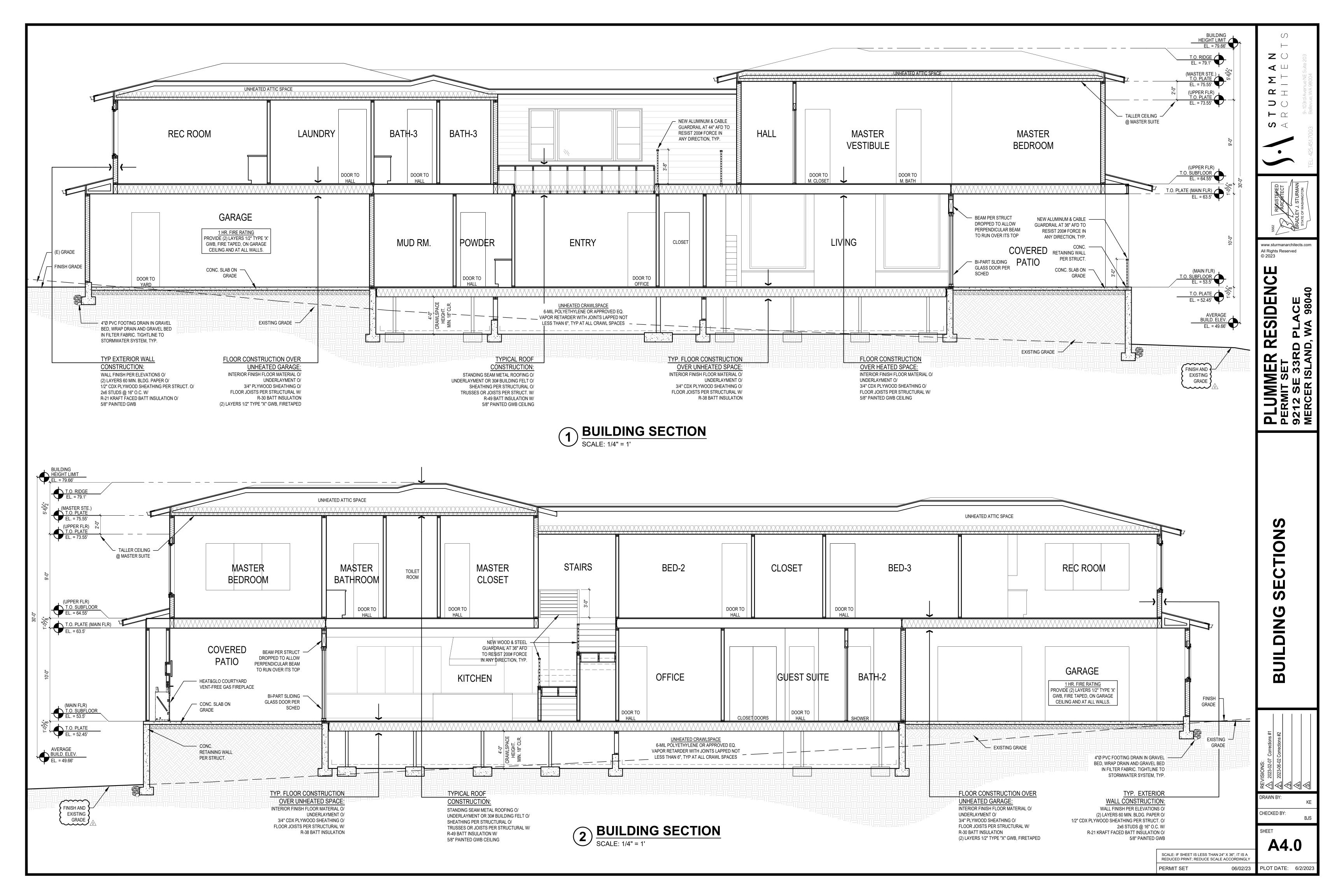


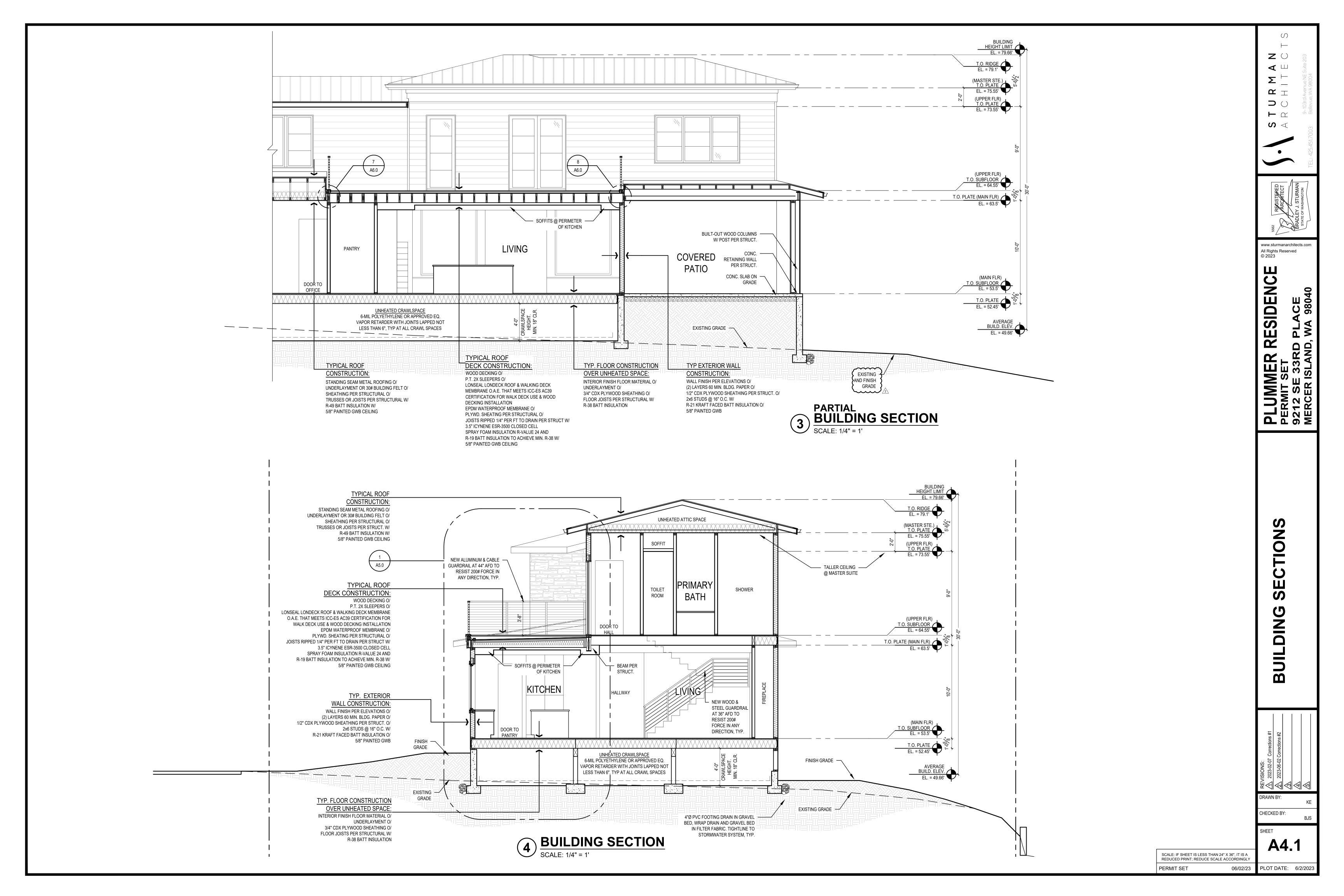


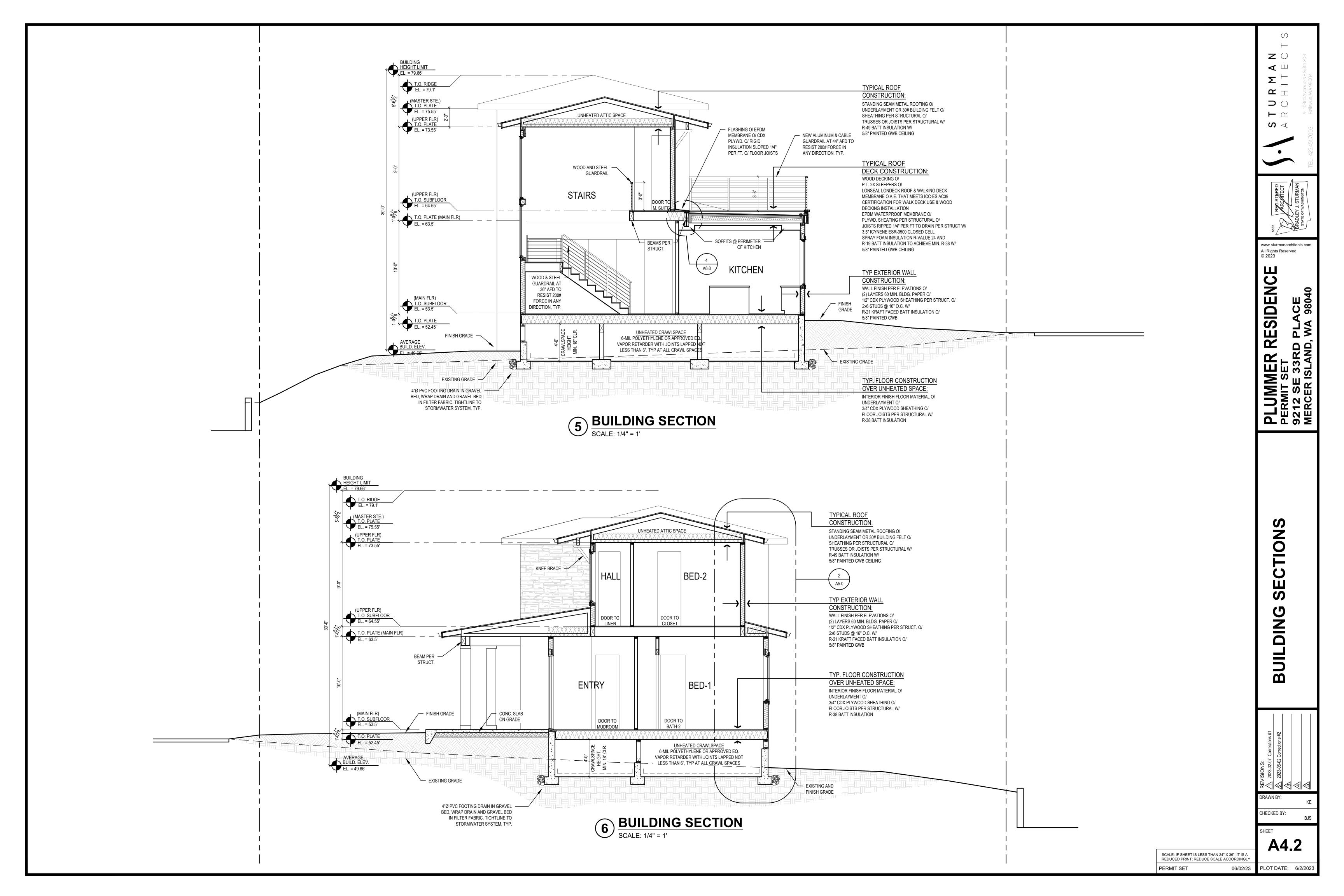


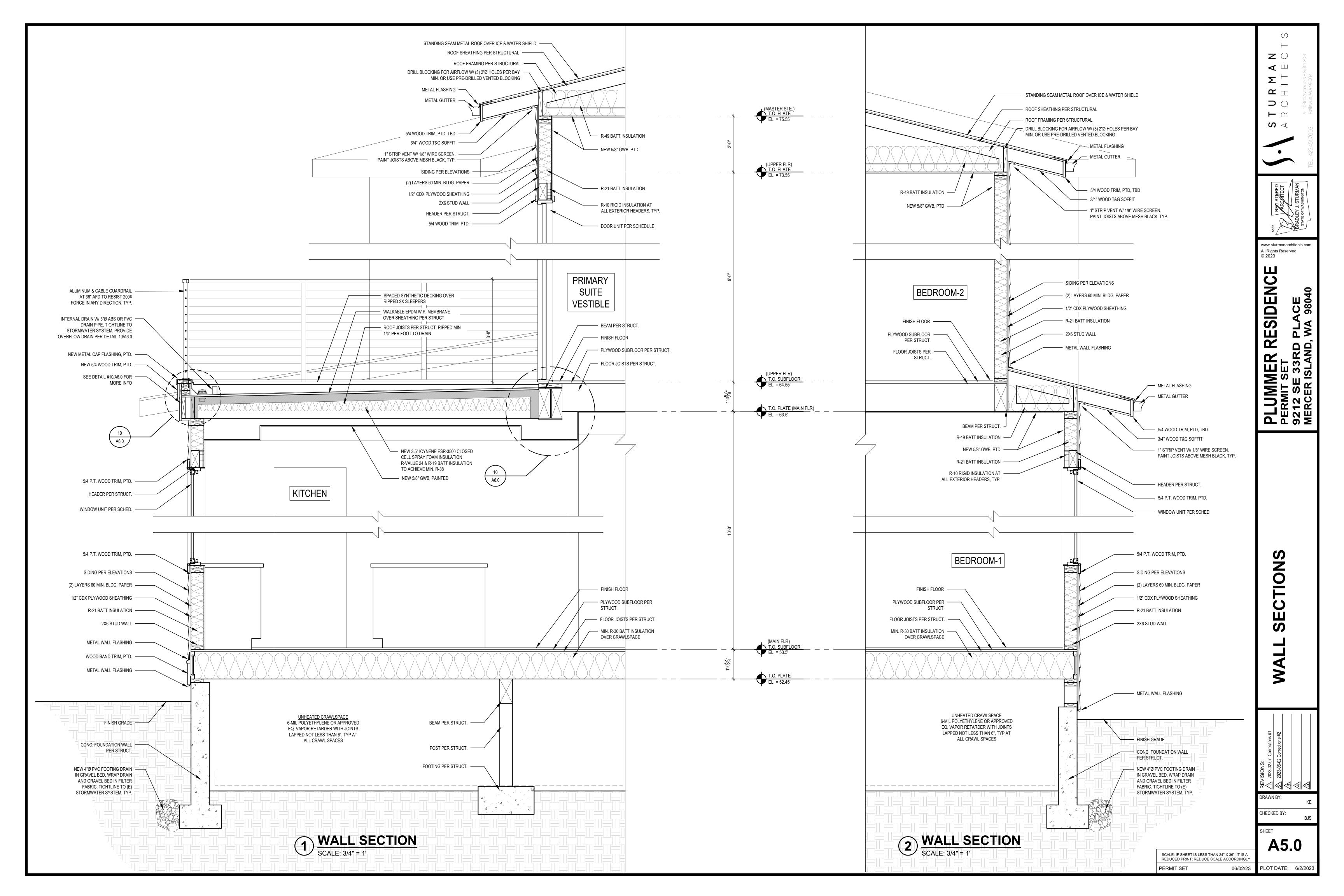


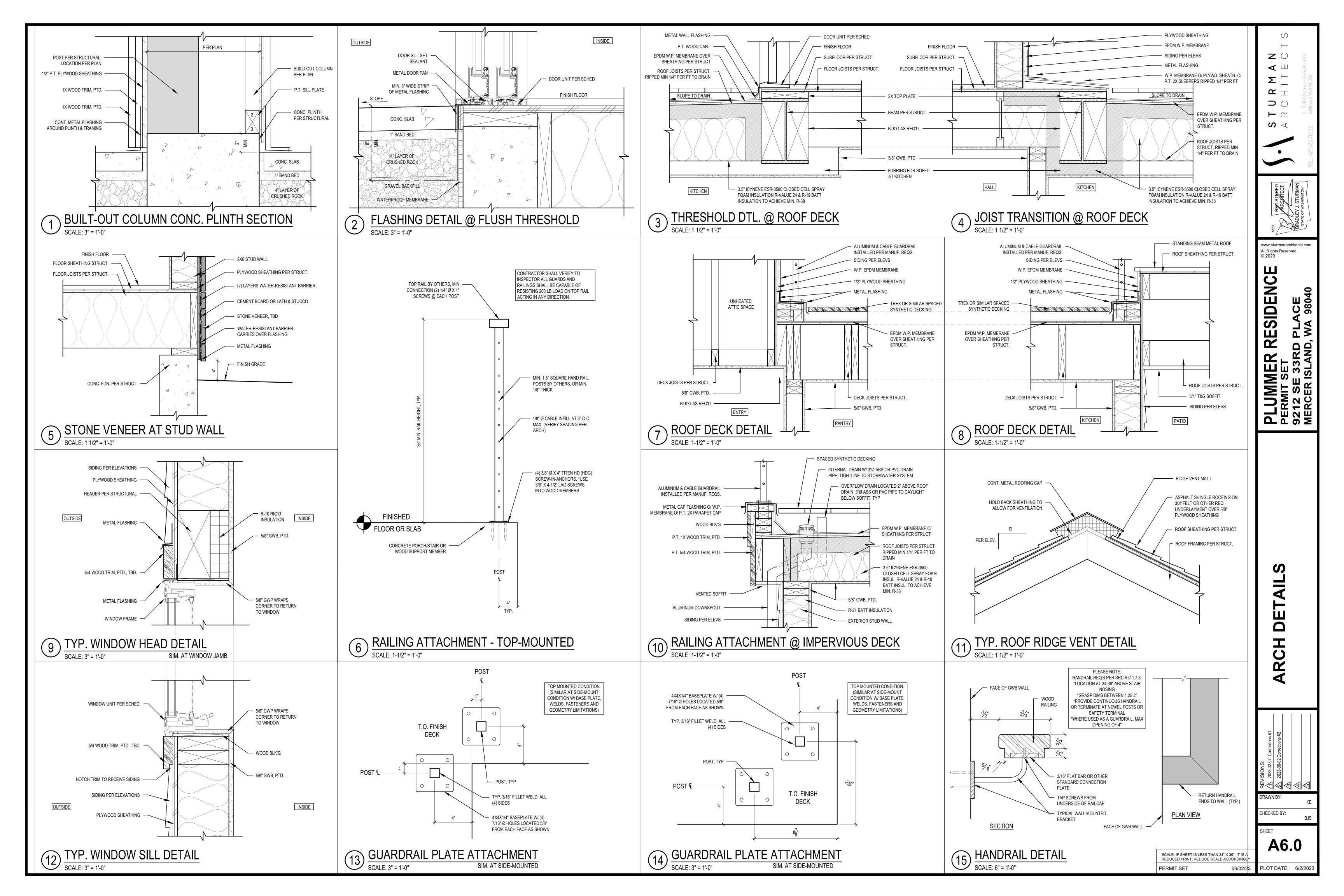












1.0 GENERAL

- 1.1 Construction shall conform to the 2018 INTERNATIONAL RESIDENTIAL CODE and all other requirements of authorities having jurisdiction.
- 1.2 These drawings are the property of O.G. Engineering, PLLC ("Engineer"). These drawings and the information contained herein shall not be used for completion of or revisions to this project by others, extensions of this project or any other project without Engineer's express written permission.
- 1.3 Refer to Architectural Plans for all dimensions and elevations not shown. Do not scale drawings. The contractor shall verify all pertinent dimensions and existing conditions prior to beginning construction. Conflicts, differences in information, and omissions in drawings shall be brought to the attention of the Engineer for resolution prior to construction. Changes from the drawings shall be made only with the prior approval of the Engineer. All work is subject to review and approval by the local building department. All work shall conform to all permit and building department requirements. All details shall be considered typical at similar conditions. Details shall be used where applicable, unless otherwise noted. Details intend to show concepts that may not exactly match specific site conditions. All work shown on these drawings is new unless noted as existing.
- 1.4 The contractor shall be solely responsible for jobsite and construction safety and compliance with all current safety regulations. Jobsite visits performed by the Engineer do not include a review of the adequacy of the contractor's safety measures. The Engineer has no authority to exercise any control over any construction contractor or their employees in connection with their work or any health or safety precautions. Only the final. permanent structure is shown on these drawings. The contractor shall be solely responsible for the means and methods of construction, including but not limited to construction sequencing and providing all necessary shoring, bracing and other temporary supports during construction. The contractor shall be solely responsible for obtaining all necessary independent engineering reviews of all temporary conditions and support systems during construction.
- 1.5 Utility information is not shown on these drawings. The contractor shall be solely responsible for locating and protecting utilities prior to and during construction. The contractor shall be solely responsible for all damage to utilities resulting from their work, and all damage to utilities shall be repaired solely at the contractor's expense.
- 1.6 All waterproofing and drainage information shown on these drawings is for illustrative purposes only. Waterproofing and drainage are the design responsibility of others.

2.0 DESIGN BASIS - BUILDING STRUCTURES

2.1 Vertical Loads (psf)	Dead	Live	Snow
Typical Roof	18*	20	25
Upper Deck	18	60	25
Typical Floor	14	40	

*Includes 4psf for solar-ready zones "Includes 4pst for solar-ready zones /1 2.2 Seismic Design Data (per the 2018 IBC) Risk Category: II

Importance Factor: le=1.0 Site Coordinates: 47.5818°N, 122.2136°W Mapped Spectral Response Acceleration: Ss=1.39, S1=0.49 Site Class: D

Spectral Response Coefficients: Sds=0.93 Seismic Design Category: D Main Seismic Force—Resisting System: Wood Structural Panel

Response Modification Factor: R=6.5 Seismic Response Coefficient: Cs=0.14 Redundancy Factor: $\rho=1.0$

Over-strength Factor: Ω =2.5 Analysis Procedure Used: Equivalent Lateral Force Procedure

2.3 Wind Design Data (per the 2018 IBC) Risk Category: II Basic Wind Speed: 98 mph

Exposure Category: C Topographic Factor: 1.00 (Per Mercer Island Wind Load Map)

3.0 INSPECTIONS

The construction work shall be inspected as required by the SRC Section R106. The contractor is solely responsible for understanding the requirements of and coordinating all inspections, observations and testing and ensuring that all work is performed to the satisfaction of the inspector.

4.0 FOUNDATIONS

4.1 New foundations have been designed in accordance with recommendations in the Geotechnical Report. The design basis is as follows:

* Allowable Vertical Bearing Pressures:

Dead + Live Dead + Live + Short Term 3325 psf * Retaining Walls:

Active Pressure 35 pcf Seismic Pressure 8H psf 300 pcf Passive Pressure Sliding Friction Coefficient

4.2 All site preparation, grading, earthwork and site drainage, including but not limited to sub-grade preparation, foundation and retaining wall excavations, structural fill specifications, compaction requirements, and site drainage installation, shall be performed in accordance with the Geotechnical Report prepared by the Geotechnical Engineer, Geotech Consultants, Inc., dated December 15th, 2020. The Geotechnical Report is part of the construction documents and a copy may be obtained from the Geotechnical Engineer's office. The contractor shall notify Geotech Consultants, Inc. (425-747-5618) in advance of any earthwork operations and Geotech Consultants, Inc. should be present to observe and test, as necessary, the earthwork and foundation installation phases of the project.

5.0 MATERIALS

5.1 Wood:

5.1.1 All untreated sawn lumber shall be Douglas Fir grade number 2. U.O.N. Mudsills and all sawn lumber in contact with concrete, masonry, ground, exposed to weather or moisture, shall be P.T. Hem Fir or Doug Fir grade number 2, U.O.N. Preservative retention levels in P.T. wood shall meet the requirements of the applicable use category in accordance with AWPA U1-16, and shall not exceed those required to comply with AWPA Use Category UC4A. Do not use wood treated with ACZA. Field-cut ends, notches and drilled holes of P.T. wood shall be treated in the field in accordance with AWPA M4. P.T. is not required at naturally decay—resistant (i.e. redwood, cedar etc.) sawn lumber members.

5.1.2 Engineered Wood Framing Members and I—Joists shall be TrusJoist® or approved equal. 'PSL' denotes Parallam 2.2E for beams and 1.8E for posts. 'LSL' denotes Timberstrand 1.55E for members with depth equal to or greater than $9\frac{1}{2}$, and 1.3E for members with depth less than $9\frac{1}{2}$ ". 'LVL' denotes Microllam 2.0E. 'TJI' denotes TJI I-joists.

5.1.3 Glulam framing members shall be DF/DF, stress class 24F-1.8E, combination symbol 24F-V8, U.O.N.

5.1.4 All wood framing members shall have 19% maximum moisture content at time of installation.

5.2 Concrete:

Hardrock, normal-weight concrete with a minimum 28-day compressive strength of 3.000 psi for concrete exposed to weather and 2,500psi for concrete not exposed to weather. Slump range shall be 3-5 inches. Maximum aggregate size shall be 1". Maximum water/cement ratio shall be 0.5. Concrete exposed to weather shall be air—entrained with total air content between 5%-7% of total concrete volume.

5.3 Reinforcing Steel Bars:

ASTM A615, Grade 60

5.4 Post-Installed Dowels & Anchors into Existing Concrete & CMU

Epoxy: Simpson SET-3G (Installed & inspected per ICC No. ESR-4057)

5.5 Bolts and Threaded Rods:

5.5.1 Threaded Rod: ASTM F1554 Grade 36

5.5.2 Sill Anchor Bolts: ASTM A307 Bent bar "J" anchor bolts shall have a hook with a 90-degree bend with an inside diameter of three bolt diameters, plus an extension of one and one half bolt diameters at the free end.

5.5.3 Bolts in Timber Connections: ASTM A307

5.5.4 Bolts in Steel Connections: ASTM A325-N (High-Strength)

5.6 Structural Steel:

Wide Flange (W): A992 (Fy = 50 ksi) Rectangular Tube (HSS): A500 Gr. B (Fy = 46 ksi) Plate and Bar: A36 (Fy = 36 ksi)

6.0 CONCRETE CONSTRUCTION

- 6.1 Concrete elements shall be constructed in single continuous pours. without construction joints, unless otherwise approved by the Engineer, Reinforcement shall be the longest lengths practical. Splices in rebar are not allowed in footings or walls less than 20 feet long. Lap splices shall be staggered at least 2 ft. in adjacent bars. Where reinforcement or anchor edge distances are noted on the drawings as "clear", the distance shall be taken from the face of reinforcement or anchor to edge of concrete. Cast—in—place reinforcement and anchor bolts shall be installed prior to concrete placement and shall not be "wet-set" into freshly poured concrete.
- 6.2 Reinforcement installation details, including rebar bends, hooks, splices and development lengths shall be in accordance with the requirements of IRC Section R608.5.4, U.O.N. Concrete materials. forms, mixing and delivery shall be in accordance with the requirements of the IRC Section R404.1.3.3.
- 6.3 Concrete Coverage over Reinforcing Steel

Unless otherwise noted, maintain the minimum concrete cover to face of reinforcement or anchors as follows:

1) 3" Where concrete is cast against and permanently exposed to earth except slab on arade. 2) 2" Where concrete is exposed to earth but formed, or exposed

3) $1\frac{1}{2}$ Where concrete is not exposed to earth or weather.

6.4 Slabs on Grade

6.4.1 Crack Control Joints

Cut crack control joints in top of slab @10'-0"o.c. (max.) each way. Joint depth shall be $\frac{1}{4}$ of the slab depth or 1", whichever is greater. Joints shall be conventional saw-cut within 4 to 12 hrs of concrete placement, or early-entry saw-cut within 1 to 4 hrs of concrete placement. Jointed panels shall be rectangular, as square as possible, with a max length—to—width ratio of $1\frac{1}{2}$:1.

6.4.2 Slab Sub-Base

Slab sub-base shall be $\frac{5}{8}$ " to $\frac{3}{4}$ " clean, crushed drain rock, compacted to a firm and unyielding condition.

7.0 WOOD CONSTRUCTION

7.1 General Framing

Connections not specified on these drawings shall conform to the IRC fastening schedule, refer to Table R602.3(1). Depth of all posts in walls shall match stud depth, U.O.N. Block floor joist space solid under posts and cripple studs supporting headers and continue support to foundation. Face nail all plies of multi-ply studs with 10d@6"o.c. Obtain approval from engineer prior to ripping or creating notches or holes in framing members, U.O.N. Install double joists below all interior walls parallel to floor joists and solid blocking below all interior walls perpendicular to floor joists, U.O.N. All beams shall be continuous across supports unless explicitly shown as multiple pieces. Install full depth blocking between framing members over supports, unless otherwise noted. Intall 2x4 blkg btwn adjacent joists/rafters/ trusses @24"o.c. over interior partitions.

7.2 Engineered Wood Framing

See TrusJoist "Installation Guide for Floor and Roof Framing" (TJ-9001) for allowable holes in engineered wood beams. Grade stamp info must be maintained on ripped engineered wood members; refer to TrusJoist Technical Bulletin TB-305 for requirements pertaining to re-sawn engineered wood.

7.3 Fasteners

Nails specified on these drawings are common nails, U.O.N. Fasteners in contact with P.T. wood, exposed to weather or in contact with ground shall be hot-dipped galvanized per SRC Section 317.3, or shall have equivalent corrosion resistance. Dissimilar metals & coatings shall not be in contact. Bolt holes shall be a minimum of $\frac{1}{32}$ to a maximum of $\frac{1}{16}$ larger than the bolt diameter. Bolts shall not be forcibly driven, and shall be tightened to the snug-tight condition. Install standard cut washers under all bolt heads and nuts bearing against wood.

7.4 Connectors

Connectors specified on these drawings are manufactured by the SIMPSON STRONG—TIE® Company. Refer to latest catalog for information not specifically noted herein. Connectors in contact with P.T. wood, exposed to weather or in contact with ground shall be ZMAX or HDG galvanized. All connectors shall receive the maximum number of fasteners, U.O.N. Dissimilar metals & coatings shall not be in contact. Shim gaps in connectors for different framing sizes with plywood as required. Non-field-adjustable hangers specified as sloped or skewed shall be manufactured sloped or skewed.

7.5 Wood Structural Panels

WSPs shall bear the APA trademark and shall meet the requirements of the latest edition of USDOC PS1 or PS2. Use 10d common wire nails to fasten panels with $1\frac{1}{2}$ " minimum penetration into framing at all panel edge and field nailing, U.O.N. Nails shall be located at least $\frac{3}{8}$ " from panel ends and edges. Stagger nails at adjoining panel edges. Drive nail heads flush with panel surface. Maintain $\frac{1}{8}$ " app between all adjoining panel edges. Center interior panel joints on framing members or blocking. Provide $\frac{1}{2}$ " space between untreated panel and concrete or masonry. Minimum panel dimension shall be 2'-0". Panel storage and handling during transport and construction shall be in accordance with APA recommendations and shall protect the panels from prolonged exposure to moisture from rain, snow, ground or other sources. WSPs permanently exposed to weather shall be exterior grade.

7.6 Shear Walls and Exterior Wall Sheathing

7.6.1 Shear walls are noted on the plans. Shear walls shall be sheathed with $\frac{1}{2}$ " APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$, U.O.N. Panels shall not be less than 4'-0" x8'-0", except at boundaries and changes in framing. Panels shall be laid with strength axis vertical. Install 2x blkg under all unsupported panel edges; all panel edges shall be supported by and fastened to min. 2x common studs or blocking, U.O.N. on shear wall schedule. Edge nail panels to posts within shear walls. Install double stud or min. 4x post at the ends of all shear walls. Provide solid blocking under double studs & posts between floors and continue support to foundation. See shear wall schedule for more information.

7.6.2 WSP Wall Nailing, U.O.N.:

Panel Edge Nailing: 10d@6"o.c. maximum. Intermediate (Field) Nailing: 10d@12"o.c. maximum.

7.6.3 All new exterior walls not called out as shear walls shall be sheathed on their exterior face with $\frac{1}{2}$ APA RATED SHEATHING. EXPOSURE 1 WSPs with a span rating of $\frac{32}{16}$ and nailing per note 7.6.2., U.O.N. All other fasteners & requirements shall conform to the shear wall schedule for wall type (1).

7.7 Holdowns and Tiedown Straps

Holdowns and tiedown straps shall be attached to double studs or min. 4x posts, U.O.N. See latest Simpson Catalog for additional requirements not noted herein. See holdown schedule for anchor bolt sizes and additional specifications. Refer to note 7.1 for nailing and framing requirements at holdown/tiedown posts. Install solid post at shear wall corners or intersections where holdowns/tiedowns occur. All holdowns/tiedowns shall have the maximum number of fasteners.

7.8 Sill Anchor Bolts

There shall be a minimum of two sill anchor bolts per piece with one bolt located not more than 12" or less than $4\frac{1}{2}$ " from each end of each piece. Holes in sills for bolts shall not be oversized. Sill anchor bolts shall be $\frac{5}{9}$ with 7" min. embed. into concrete. Sill anchor bolts into existing concrete shall be all—thread rod, drill and epoxy. See shear wall schedule for spacing of sill anchor bolts in shear walls. Maximum sill anchor bolt spacing at non-shear-walls shall be 6'-0"o.c. at interior walls and 4'-0"o.c. at exterior walls. All sill anchor bolts at shear walls and mudsills shall be installed with 0.229"x3"x3" steel plate washers. Edge of sill anchor bolt plate washers shall be located $\frac{1}{2}$ " max, from inside face of wall sheathing or rim joist where occurs.

7.9 Floor and Roof Sheathing

7.9.1 Wood structural panel sheets at floors and roofs shall be laid with strength axis perpendicular to supports and continuous over two or more spans, unless otherwise noted on drawings. Stagger adjacent panels 4'-0"o.c. lengthwise.

7.9.2 Unless otherwise noted, typical roof sheathing shall be unblocked §" APA RATED SHEATHING, EXPOSURE 1 WSPs with a span rating of $\frac{40}{20}$. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. intermediate (field) nailing. Install 'PSCL' sheathing clips (one mid-way between each support) at all unsupported panel

7.9.3 Unless otherwise noted, typical floor sheathing shall be unblocked $\frac{3}{4}$ " APA RATED STURD-I-FLOOR EXPOSURE 1 WSPs with a span rating of $^{48}/_{24}$ and T&G edges. Panels shall be fastened to framing members with 10d nails @6"o.c. at all supported panel edges and 10d nails @12"o.c. field nailing. Glue sheathing to all supports (including blocking) with $\frac{1}{4}$ " minimum beads of approved adhesive meeting APA specification AFG-01.

7.10 Metal-Plate-Connected Wood Trusses

7.10.1 The design, manufacture and installation of trusses shall be in accordance with the requirements of ANSI/TPI 1 and the IRC Section R502.11.

7.10.2 Trusses, structural fascia, their connections to other trusses/fascias, and truss eave blocking are the design responsibility of the supplier, and shall be designed by a civil or structural engineer licensed in the State of Washington ("Truss Designer"). Trusses shall be designed to support the following specific unfactored loads in addition to their self-weight:

<u>Vertical Roof Loads — Top Chord</u> *Dead: 14 psf (Does not include truss self-weight)

*Live: 20 psf *Snow: 25 psf

*Wind: -51 psf (uplift)

<u>Vertical Ceiling Loads — Bottom Chord</u> *Dead: 5 psf (Does not include truss self-weight) *Live: 10 psf (Does not act concurrently with roof live load)

<u>Lateral Drag Truss Loads — Bottom Chord</u> *Seismic: 3410 lbs (total)

(required at each truss indicated with "DTR" on the roof framing plans. Load acts parallel to bottom chord, distributed uniformly along "lap" length of with shear walls(s) below; refer to plans and details for attachment to shear walls below).

7.10.3 Trusses shall not rely on interior walls for support, U.O.N.: trusses shall be designed to span between exterior bearing walls.

7.10.4 Trusses shall be braced to provide lateral stability and prevent rotation in accordance with the SBCA BCSI "Guide to Good Practice for Handling, Installing and Bracing of Metal-Plate-Connected Wood Trusses". Bracing shall be designed and specified by the truss designer.

7.10.5 Trusses and their connections shall not be notched, cut, spliced or otherwise altered or damaged in any way without the prior written consent of both the E.O.R. and truss designer.

7.10.6 Truss design drawings and calculations, prepared by a civil or structural engineer licensed in the State of Washington in accordance with the SRC Section R502.11.4, shall be submitted to the contractor, architect, engineer and local building official for review and acceptance prior to fabrication, and shall be provided with the shipment of trusses to the job site.

7.10.7 Attach top plates of interior, non-bearing partition walls to truss bottom chords with 'STC' clips, leaving a $\frac{1}{4}$ " to $\frac{1}{2}$ " vertical gap between bottom of truss and top of plate. Attach adjacent gypsum board ceiling to top plate with 'DS' clips. Do not fasten gypsum board ceiling to truss bottom chord within 16" of top plate.

8.0 STRUCTURAL STEEL

8.1 Steel fabrication and erection shall be in accordance with "Specification for Structural Steel Buildings" (AISC 360-10).

8.2 Welding shall be in accordance with "Structural Welding Code - Steel" (AWS D1.1, latest edition) Specifications. Minimum tensile strength of weld metal shall be 70 ksi, U.O.N. Welding electrodes shall be as recommended by their manufacturer for the position and other conditions of actual use. All welding shall be performed by AWS Certified Welders.

8.3 Bolt holes shall be drilled or punched. Bolt holes shall be standard, and hole size shall be $\frac{1}{16}$ " larger diameter than the nominal size of bolt used, U.O.N. Bolts shall be installed snug-tight, U.O.N.

8.4 All steel framing and fasteners exposed to weather or in contact with ground shall be hot-dipped galvanized after fabrication to meet the requirements of ASTM 153. Upon completion of erection; touch-up, de-slag, clean and apply zinc-rich primer to exposed welds or other unprotected markings incurred during the transportation, handling or erection process. Dissimilar metals & coatings shall not be in contact.

8.5 No penetrations shall be made through steel framing except with the prior written permission of the engineer.

8.6 Structural steel shop drawings shall be submitted to the architect and engineer for review and acceptance prior to fabrication.

ABBREVIATIONS

ADJACENT ALT. ALTERNATE

ARCH. ARCHITECT A.T.R. ALL-THREAD ROD

B.F. BALLOON-FRAMED BLKG BLOCKING

BLW. BELOW

ВМ BEAM BOTT. воттом

CAST-IN-PLACE C.I.P. C.J. CONSTRUCTION JOINT

CL CENTERLINE CLR. CLEAR

CONT CONTINUOUS

CSK. COUNTERSINK

DIAMETER

DBL. DOUBLE

DF DOUGLAS FIR

DIM DIMENSION

D.J. DOUBLE JOIST D.R. DOUBLE RAFTER

E.J. **EXPANSION JOINT**

ELEV. ELEVATION

EMBED. EMBEDMENT ENGR. **ENGINEER**

E.N. EDGE NAILING E.O.R. ENGINEER OF RECORD

EQ. EQUAL

E/W EACH WAY **EXISTING**

(E) F.J. FLOOR JOIST

F.N. FIELD NAILING FOOTING FTG

G.L. GRID LINE

GLB GLULAM BEAM G.C. GENERAL CONTRACTOR

HOT-DIPPED GALVANIZED HDR HEADER

HEM FIR

2018 INTERNATIONAL BUILDING CODE® INVERTED

IRC 2018 INTERNATIONAL RESIDENTIAL CODE®

KILN-DRIED LUMBER

LOCN LOCATION

MAXIMUM

M.B. MACHINE BOLT

MIN. MINIMUM

MANUFACTURER

NOT SHOWN FOR CLARITY NOT TO SCALE

0/ OVER

ON CENTER o.c. O/H OPPOSITE HAND

OPNG OPENING

PLPLATE

PSF POUNDS PER SQUARE FOOT РΤ PRESSURE-PRESERVATIVE-TREATED

QUADRUPLE

REQUIRED

RFT RETROFIT R.R. ROOF RAFTER

R.W. REDWOOD

SEE ARCHITECTURAL DRAWINGS

S.O.G. SLAB ON GRADE SIM. SIMILAR

SQ. SQUARE STD STANDARD

SHEAR WALL SCHEDULE

TO BE DETERMINED

TOP & BOTTOM T&G TONGUE & GROOVE

TYP. TYPICAL

TRPL.

T.O. TOP OF

TRIPLE

UNLESS OTHERWISE NOTED U/S

UNDERSIDE UNDER

VERIFY IN FIELD

WESTERN RED CEDAR WATERPROOFING W.P.

WOOD STRUCTURAL PANEL

186 NA)S .UMMER PI 98040

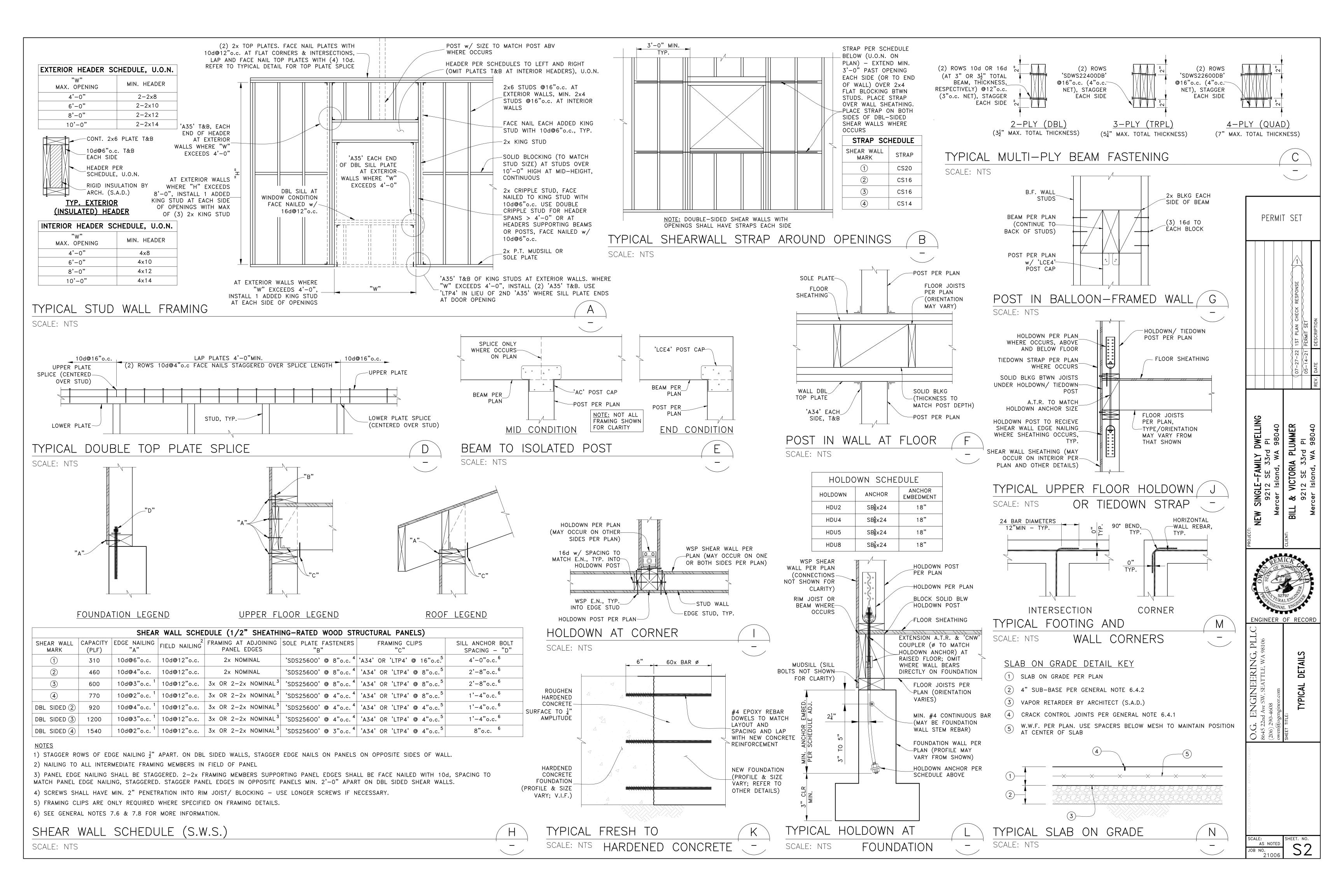
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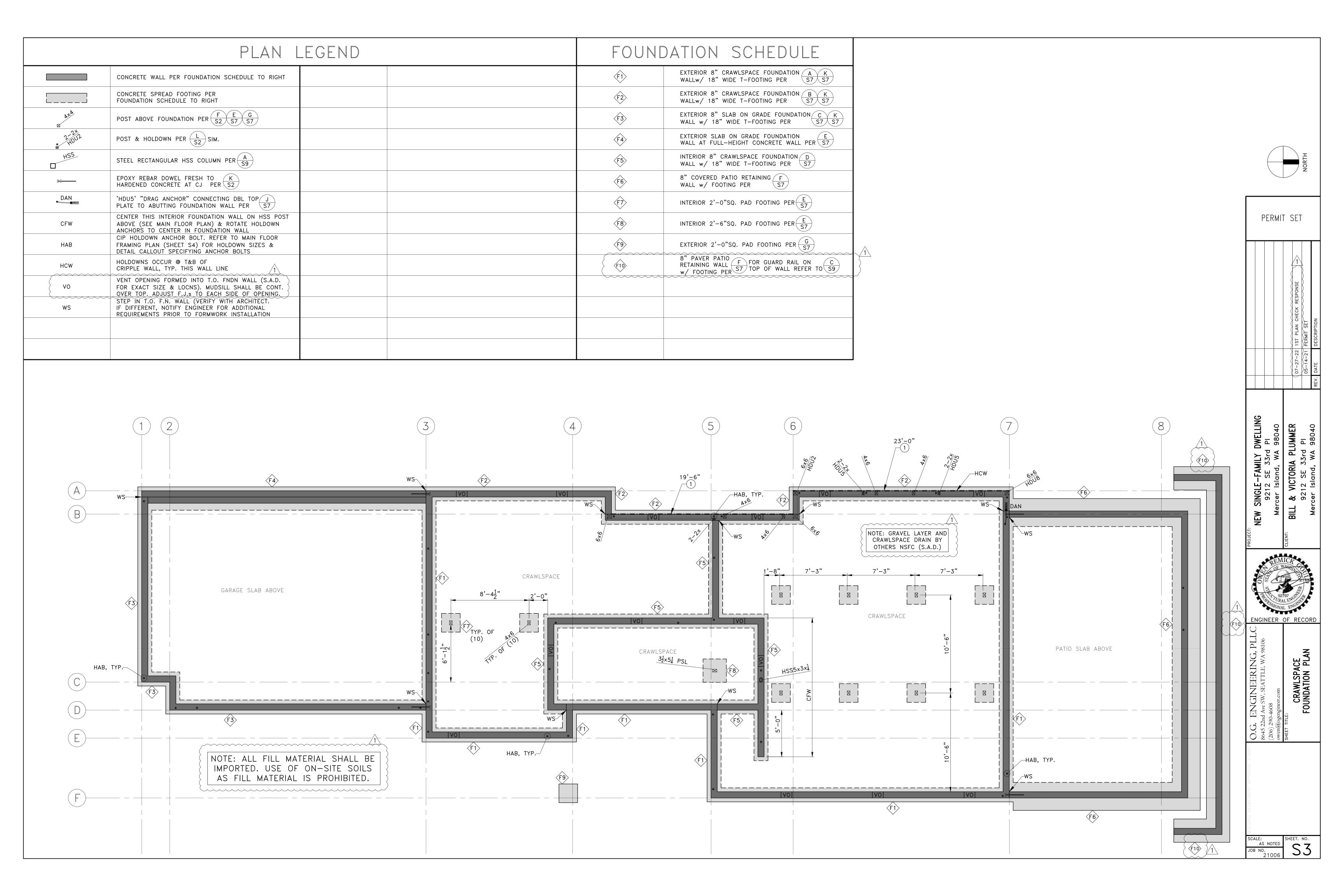
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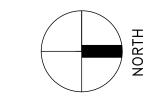
PLL NGINEERING, Ave SW, SEATTLE, WA 9 NOTES DETAIL GENERAL TYPICAL

ENGINEER OF RECORD

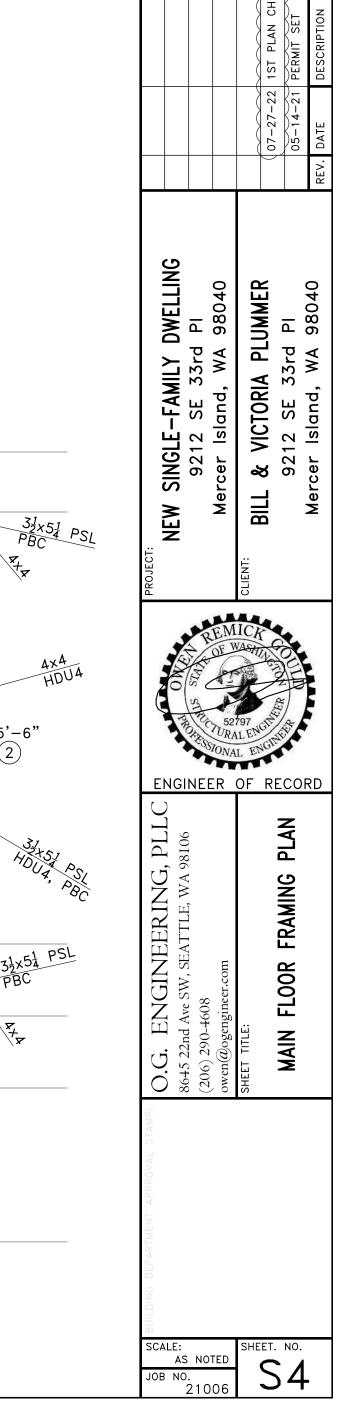


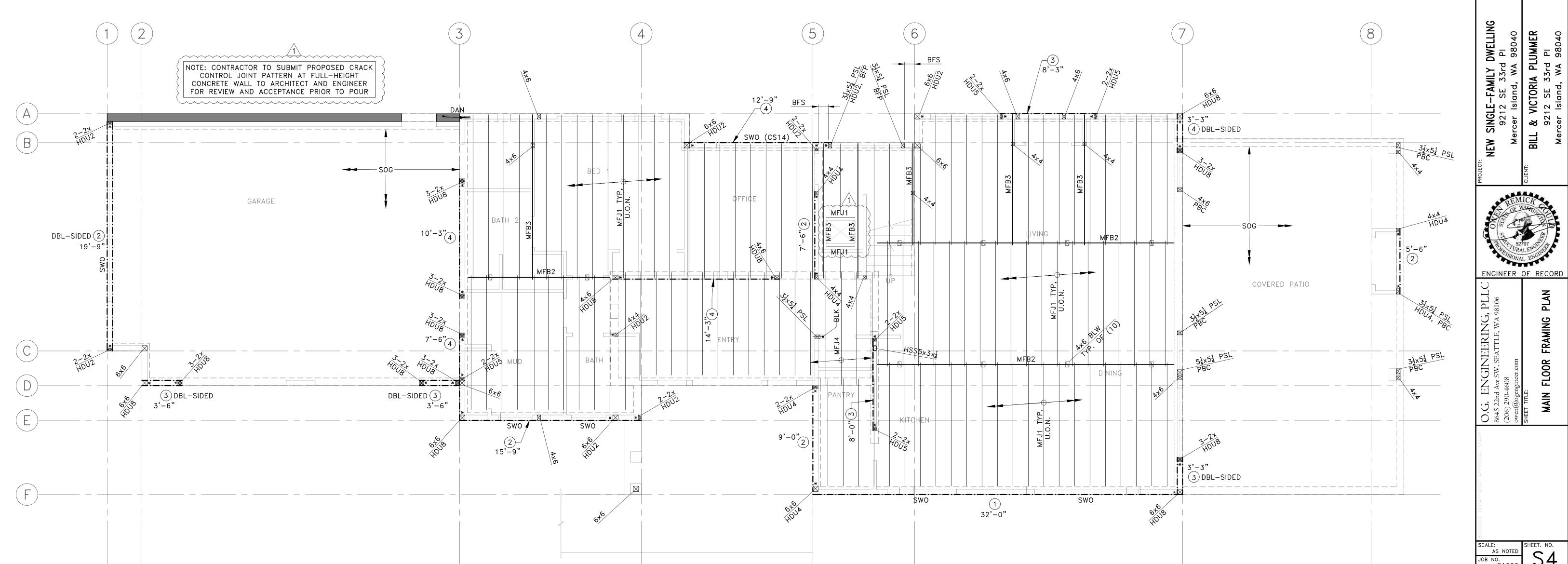


	PLAN L	EGEND			FRAMING	SCHE	DULE
	FULL-HEIGHT CONCRETE WALL PER E S9	DAN	'HDU5' "DRAG ANCHOR" CONNECTING DBL TOP J PLATE TO ABUTTING FOUNDATION WALL PER S7	CALLOUT	JOIST/BEAM	HANGER	REFER TO DETAIL(S)
	STUD WALL ABOVE FLOOR	DDO	POST SHALL HAVE 'ABUZ' BASE (SIZE TO MATCH POST) BEARING DIRECTLY ON TOP OF FNDN STEM WALL w/ 5" of FPOXY ANCHOR w/ 12" MIN FMRFD	MFJ1	11 ⁷ / ₈ TJI 360 @16"o.c.	MIT3511.88	A-B D-E S7 S7
	WALL BELOW FLOOR	PBC	WALL w/ 5"ø EPOXY ANCHOR w/ 12" MIN. EMBED. INTO CONCRETE. PACK MIN. 7000psi NON-SHRINK, NON-METALLIC GROUT SOLID UNDER 1" POST BASE STANDOFF PLATE PRIOR TO INSTALLATION	MFB2	$5\frac{1}{2} \times 10\frac{1}{2}$ GLB (DROPPED)	N/A	E S7
	WINDOW BY ARCH (S.A.D.)	SOG	4" CONCRETE SLAB ON GRADE N w/ 6x6-W4.5xW4.5 W.W.F. PER S2	MFB3	$3\frac{1}{2} \times 11\frac{7}{8}$ LSL (FLUSH)	N/A	N/A
X 'L'	1" W.S.P. SHEAR WALL TYPE X H A-D A-B M A W/ MIN. LENGTH 'L' PER S2 S7 S8 S8 S9	SWO (ALT. STRAP)	STRAP AROUND OPNGS IN SHEAR WALL PER (USE ALTERNATE STRAP IF INDICATED IN PARENTHESES ON PLAN)	MFJ4	1¾x11½ LVL @16"o.c. (BLW WALL SUPPORTING HOT TUB)	N/A	D S7
OR ATA	POST ABOVE <u>OR</u> BELOW FLOOR PER <u>E-G E G</u> S7 S7						
* 2,1012	POST & HOLDOWN PER J L SIM.						
HSS	STEEL RECTANGULAR HSS COLUMN PER S9						
BLK	$3^1_2 \times 11^7_8$ LSL SOLID BLKG BTWN JOISTS BLW MAIN FLOOR POST/ ABOVE CRAWLSPACE POST						
BFP	B.F. POST FROM MAIN FLOOR TO ROOF w/ 'A35' EACH SIDE, T&B						
BFS	1 ³ / ₄ ×5 ¹ / ₂ LVL STUDS @16"o.c. w/ 'A35' T&B, B.F. FROM MAIN FLOOR TO ROOF (ADJ. TO MAIN FLOOR STAIR OPNG)	BEAM_ HANGER	FLUSH-FRAMED JOIST OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON PLAN OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)				
		5	JOIST OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT SHOWN FOR CLARITY). INSTALL FULL—DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT				



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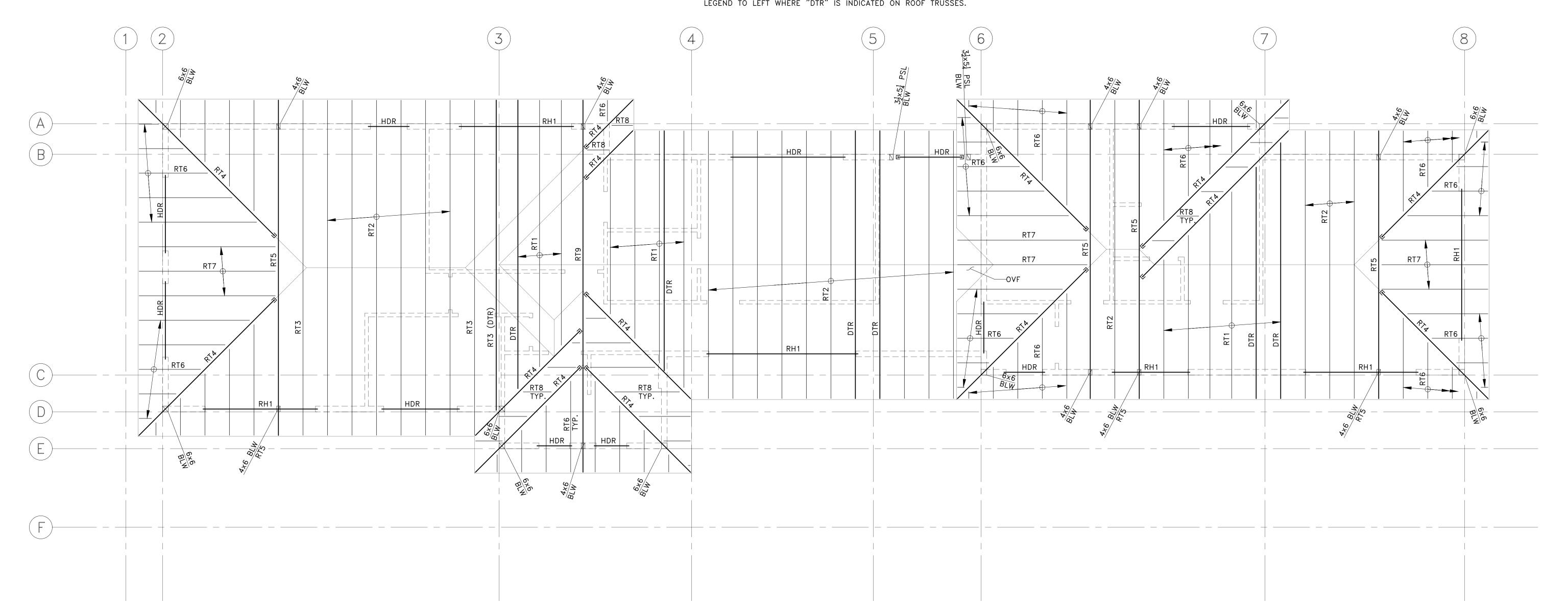


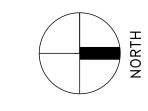


The content of the	AMING SCHEDULE STRUCTURAL FASCIA MEMBERS OTHER TRUSSES/FASCIAS ARE	*ALL METAL-PLATE CONNECTED WOOD TRUSSES, STRUCTURAL FASCIA MEMBERS AND CONNECTIONS TO OTHER TRUSSES/FASCIAS ARE DESIGN-BUILD BY TRUSS	
March Marc	TO DETAIL (C) VARY BETWEEN TRUSSES AND I	FASCIAS OF THE SAME	
The content of the	SIM N	ERIA AND OTHER INFO.	
The content of the	LRT1* LOW ROOF TRUSSES @24"o.c. BY SUPPLIER J SHALL RELY ON D-LINE BEARING WALL FOR SUPPORT		
March Marc			
Company Comp	SIM H-I	IORTH	
The content of the	LRB4 $5\frac{1}{2} \times 10\frac{1}{2}$ GLB N/A SIM H S8	Z	
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PLAN LEGEND					FRAMING SCHEDULE			STRAP SCHEDULE (REFER TO SHEET S5 FOR STRAP LOCNS)	
	WALL BELOW FLOOR			CALLOUT	JOIST/BEAM	HANGER (U.O.N. ON PLAN)	REFER TO DETAIL(S) (OR SEE NOTES BLW)	ST1	'LSTA30' STRAP o/ FLOOR SHEATHING o/ 1ST JOIST ADJ. TO G.L. 5 SHEAR WALL RIM JOIST. (ADD JOISTS AS REQ'D TO ALIGN w/ STRAPS)
A+AN BLW	POST BELOW ROOF PER E-F			RT1*	IRREGULAR COMMON TRUSSES @24"o.c.	BY SUPPLIER	K S7	ST2	'LSTA30' STRAP U/S OF FLOOR BEAM/JOIST TO UDJ1 (ADD UDJ1 AS REQ'D TO ALIGN w/ STRAP)
	METAL STRAP PER PLAN			RT2*	COMMON GABLE TRUSSES @24"o.c.	N/A	K S7	ST3	'MSTC40' STRAP o/ WALL SHEATHING OUTSIDE A OF DBL TOP PLATE ACROSS UFB10 PER S9
				RT3*	STEP DOWN TRUSS	N/A	K S7	ST4	'CS14'x6'-0" STRAP OUTSIDE OF WALL SHEATHING o/ B.O. HDR TO 2x4 FLAT BLKG BTWN SHEAR WALL STUDS PER
				RT4*	HIP JACK TRUSS	BY SUPPLIER	K S7	ST5	'MSTA30' STRAP o/ FLOOR SHEATHING o/ T.O. ABUTTING BEAMS
				RT5*	HIP GIRDER TRUSS	BY SUPPLIER	K S7	ST6	'CS16' STRAP o/ FLOOR SHEATHING, LAP MIN. 18" o/ BEAM AND CONTINUE o/2x4 FLAT BLKG ACROSS FLOOR JOISTS
				RT6*	CORNER JACK TRUSSES @24"o.c.	BY SUPPLIER	K S7	ST7	'MSTA30' U/S BEAM TO T.O. DBL TOP PLATE PER S9
DTR	DRAG TRUSS CARRYING LATERAL LOADS; ATTACH TO SHEAR WALL BELOW PER L SUPPLIER SHALL DESIGN TRUSS FOR S8 "LATERAL DRAG TRUSS LOADS" SPECIFIED			RT7*	END JACK TRUSSES @24"o.c.	BY SUPPLIER	K S7	ST8	(2) 'CS16' STRAPS (SIDE BY SIDE) o/ FLOOR SHEATHING, LAP MIN. 36" o/ BEAM OR RIM JOIST AND CONTINUE o/2x4 FLAT BLKG ACROSS FLOOR JOISTS
	"LATERAL DRAG TRUSS LOADS" SPECIFIED ON SHEET S1, GENERAL NOTE 7.10.2 IN ADDITION TO SPECIFIED VERTICAL LOADS			RT8*	PARTIAL HIP TRUSSES @24"o.c.	BY SUPPLIER	K S7	ST9	'MSTC66' U/S BEAM TO T.O. DBL TOP PLATE PER S9
HDR	DROPPED HEADER OVER WALL OPNG BLW PER USE 'HUC' HANGER (DEPTH TO MATCH HEADER) TO FULL—HEIGHT POST WHERE OCCURS			RT9*	GIRDER TRUSS	N/A	K S7	ST10	'CS16' STRAP o/ FLOOR SHEATHING, LAP MIN. 36" o/ ADDED 14 LSL BLKG SISTERED INSIDE OF RIM JOIST w/
OVF	TRUSS OVER-FRAMING BY TRUSS SUPPLIER	BEAM_ HANGER	FLUSH-FRAMED JOIST/TRUSS OR BEAM CONNECTION; SEE FRAMING SCHEDULE FOR HANGERS, U.O.N. ON	RH1	(2) 1 ³ / ₄ ×11 ⁷ / ₈ LVL (DROPPED HEADER)	N/A	A S2	((2) STAGGERED ROWS 16d@3"o.c. NET AND CONTINUE o/2x4 FLAT BLKG ACROSS FLOOR JOISTS
			PLAN OR DETAILS (JOIST HANGERS NOT SHOWN ON PLAN FOR CLARITY)					ST11	'MSTA30' STRAP o/ T.O. BEAM TO T.O. DBL TOP PLATE
		<u> </u>	JOIST/TRUSS OR BEAM BEARING ON DROPPED BEAM OR HEADER (BEARING WALL SIM). POST DOWN TO HEADER WHERE OCCURS (POST WIDTH TO MATCH BEAM, NOT					ST12	'CS20' STRAP o/ FLOOR SHEATHING, LAP MIN. 18" o/ BEAM AND CONTINUE o/2x4 FLAT BLKG ACROSS FLOOR JOISTS
		SHOWN FOR CLARITY). INSTALL FULL—DEPTH BLKG EACH SIDE OF JOIST OR BEAM OVER SUPPORT		LATE CONNECTED WOOD			ST13	'CMSTC16' STRAP o/ WALL SHEATHING OUTSIDE FACE OF SHEAR WALL DBL PLATE PER S9	

*ALL METAL-PLATE CONNECTED WOOD TRUSSES, STRUCTURAL FASCIA MEMBERS, THEIR CONNECTIONS TO OTHER TRUSSES/FASCIAS AND TRUSS EAVE BLKG ARE DESIGN-BUILD BY TRUSS SUPPLIER. DIMENSIONS, SPANS AND SUPPORT CONDITIONS VARY BETWEEN MEMBERS OF THE SAME CALLOUT (S.A.D). REFER TO SHEET S1, GENERAL NOTE 7.10 FOR TRUSS DESIGN CRITERIA AND OTHER INFO. SEE PLAN LEGEND TO LEFT WHERE "DTR" IS INDICATED ON ROOF TRUSSES.

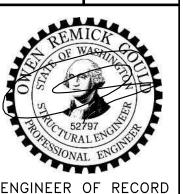




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					07-27-22	05-14-21	REV. DATE	
							REV.	

NEW SINGLE-FAMILY DWELLING
9212 SE 33rd PI
Mercer Island, WA 98040

BILL & VICTORIA PLUMMER
9212 SE 33rd PI
Mercer Island, WA 98040



ENGINEER OF RECOR

Co. C. CINCLINEERING, F.E. 645 22nd Ave SW, SEATTLE, WA 98106 206) 290-4608

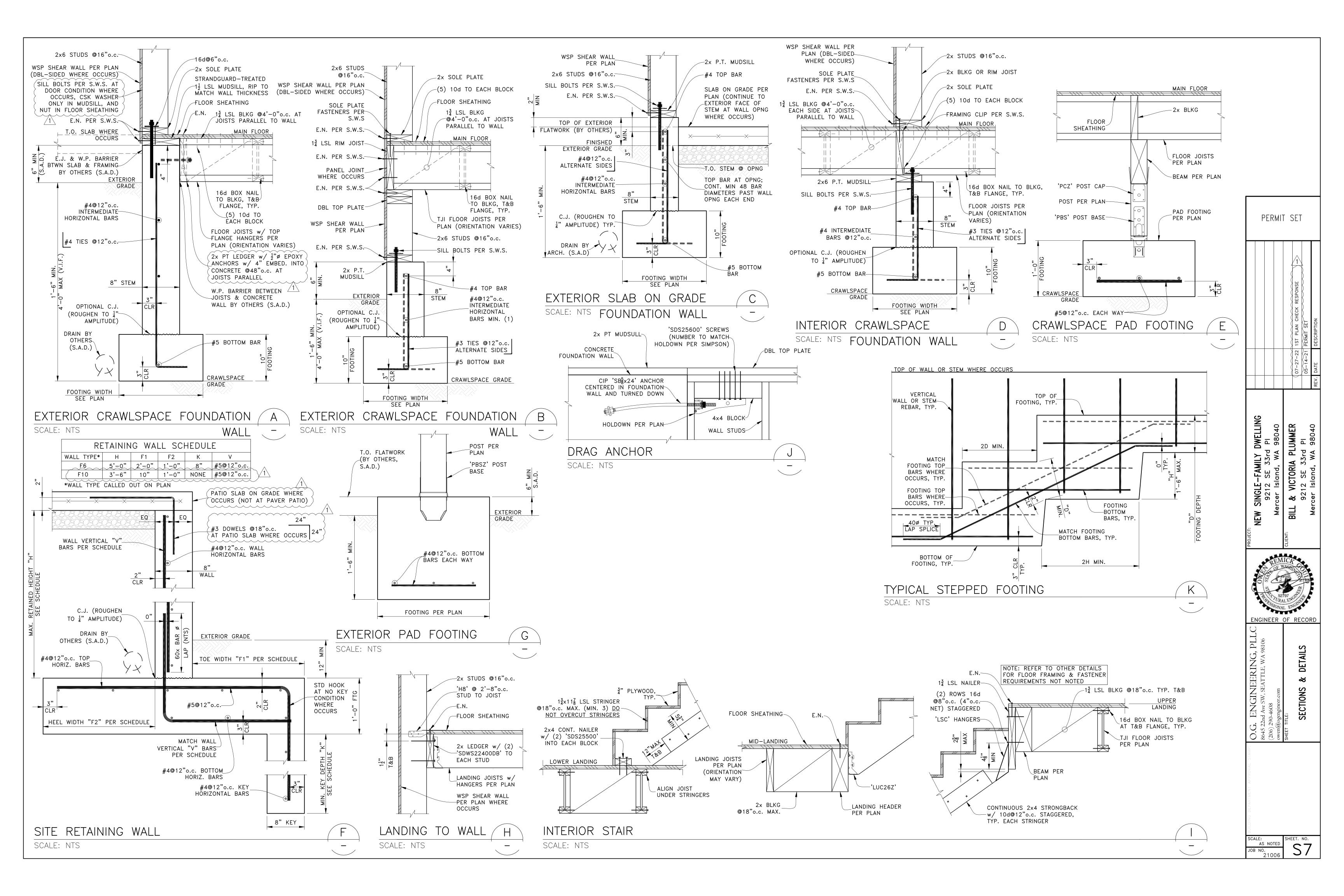
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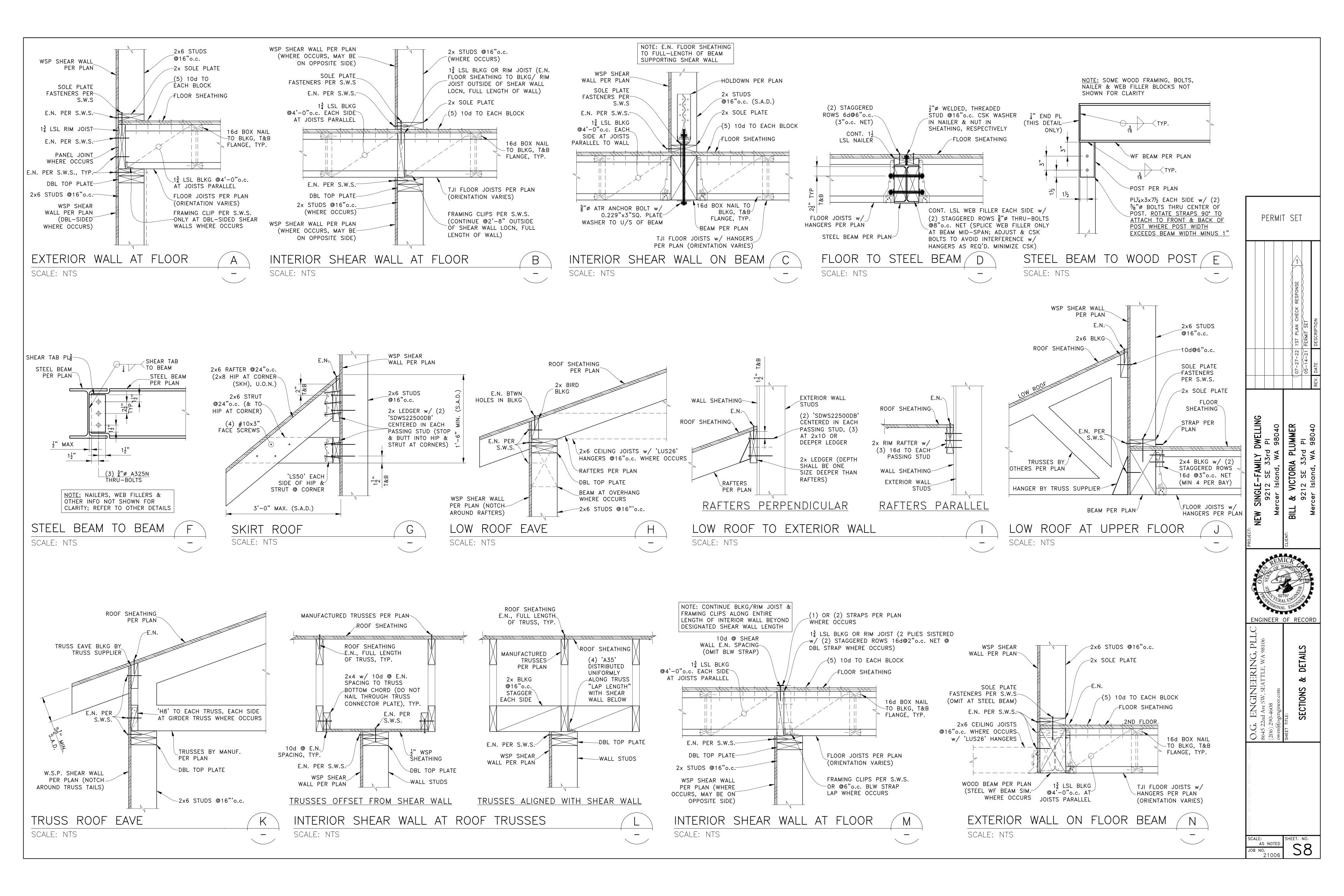
ROOF FRAMING PLAN

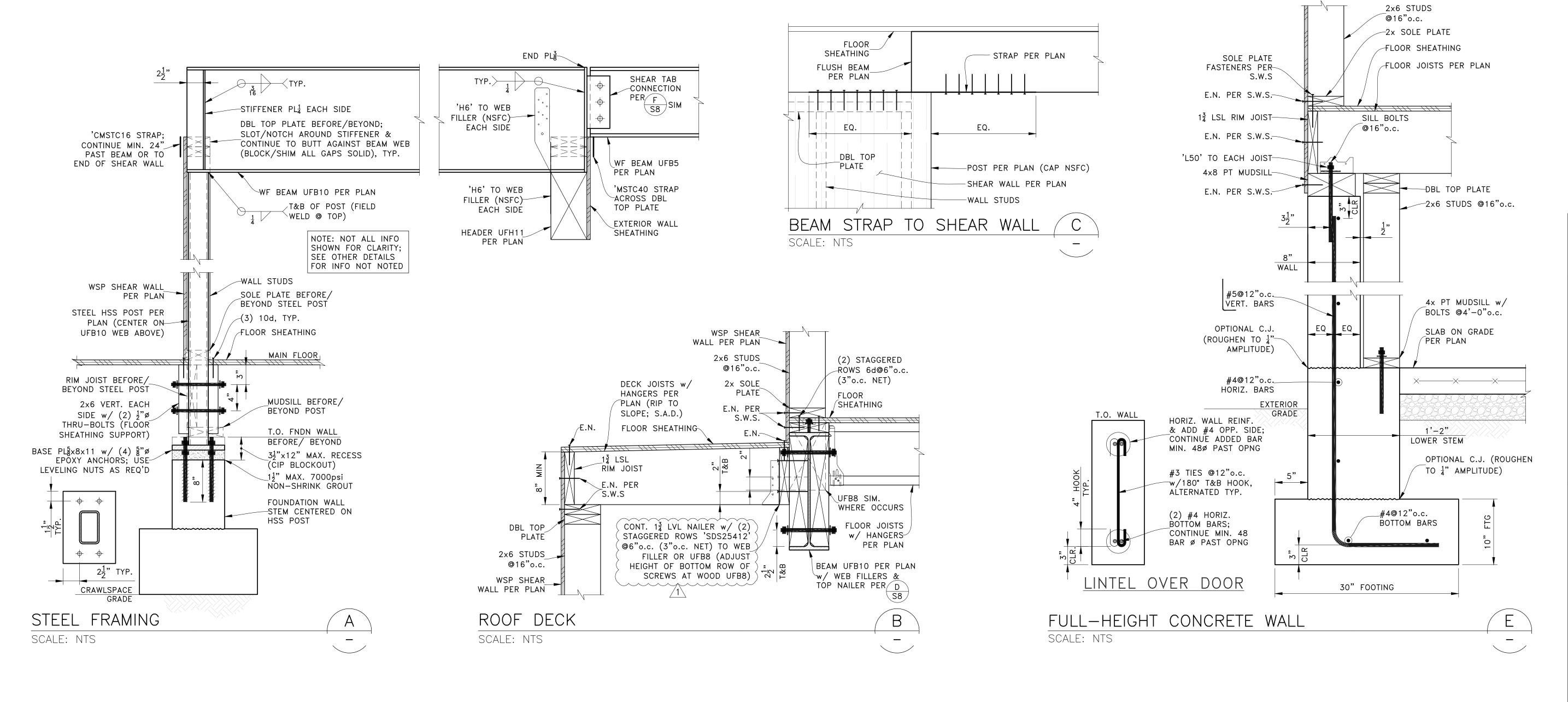
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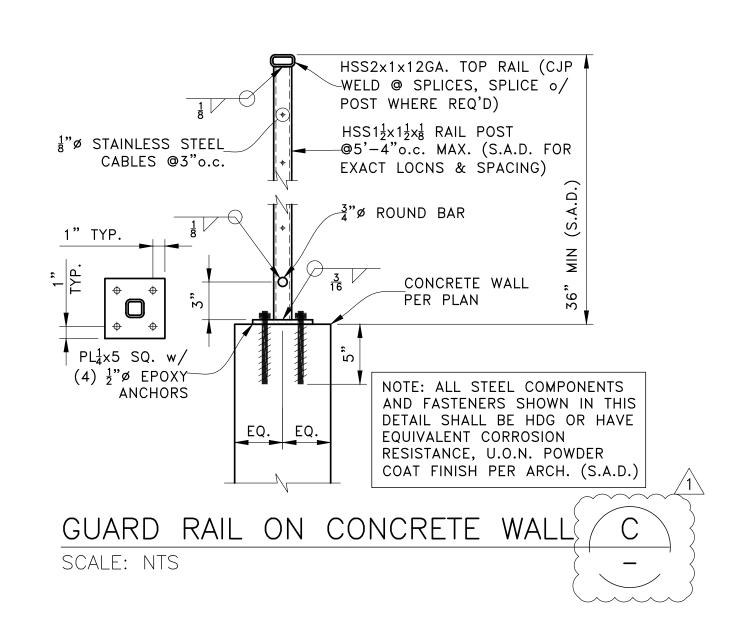
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9212 SE 33rd
Mercer Island, WA
BILL & VICTORIA PLL
9212 SE 33rd
Mercer Island, WA

ENGINEER OF RECORD

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